GENERAL NOTES

Except as shown in the plans, structure excavation and backfill shall be in Accordance with M-206-1 for cast-in-place retaining walls or M-206-2 for bridges.

Structure excavation and backfill shall be as shown on the plans, except shoring may be required for excavation adjacent to the existing roadway. Temporary excavation support shall be paid for by Item 206 Shoring. Incidental shoring that is not included as a pay item will not be measured and paid for separately but shall be included in

Expansion joint material shall meet AASHTO Specification M213.

A colored Structural Concrete Coating finish will be required, as shown on the plans, on exposed concrete surfaces. The color shall be xxxx, equivalent to AMS Standard 595 Color No. xxxx, and is to be selected from test panels provided by the Contractor

The final finish for the surfaces of the Type 9 bridge rail and Type 10 curbs shall be Class 2. All other exposed concrete surfaces shall receive a Class 1 final finish to one foot below the ground line.

All structural steel not otherwise noted shall be painted in accordance with Section 509 of the Standard Specifications. The color shall be xxxx, equivalent to AMS Standard 595 Color No. xxxx.

Except as noted below, all AASHTD M270 Grade 50 (ASTM A588) steel shall not be painted. The unpainted steel shall be cleaned in accordance with Section 509 of the Standard Specifications. Under expansion devices, the end 5 feet of the girders (inside and outside of box girders), along with all girder attachments (including diaphragms) in the 5 foot area, shall be painted in accordance with Section 509 of the Standard Specifications. The color shall be xxxx, equivalent to AMS Standard 595 Color No. xxxx.

The following structural steel shall be AASHTO M270 Grade 36 (ASTM A36): diaphragms, lateral bracing, splice plates, expansion devices, bearing plates, bearing devices, bridge railing post anchor, and stiffeners.

The following structural steel shall be AASHTO M270 Grade 50 (ASTM A572): girders, splice plates, piling, and bridge railing posts, plates & splices.

AASHTO M-222 (ASTM A588) may be substituted for M270 Grade 50 (ASTM A572) at no additional cost to the project.

All bolts shall be $\frac{1}{8}$ " diameter, high strength, unless otherwise noted.

Field welding of any kind shall not be permitted on the steel girders unless specifically called for in the plans.

Leveling pads are unlaminated bearings. They shall be cut or molded from AASHTO elastomer grade $3,\,4,\,$ or 5 as described in tables 705-1 and 705-2 with a durometer (Shore "A") hardness of 60.

Grade 60 reinforcing steel is required.

All reinforcing steel shall be epoxy coated unless otherwise noted.

All the provisions for bridge deck concrete shall also apply to approach slab concrete.

An emergency deck construction joint may be located at the one quarter span point back from a pier or abutment with respect to the direction of the deck placement.

The Contractor shall be responsible for the stability of the structure during construction. Designer/detailer: Use One Only.

Permanent Deck Forms are required.

Permanent Deck Forms are optional.

Designer/detailer: Use first Permanent Steel Deck Forms are not allowed. Stations, Elevations note Ifor widening or modifiying For structure number installation, see Standard S-614-12. lexisting structures

Stations, Elevations, and Dimensions contained in these plans are calculated from the "As Constructed Plans". These Stations, Elevations, and Dimensions may be adjusted to meet the existing structure. The Contractor shall verify all dependent dimensions in the field before ordering or fabricating any material.

Stations, Elevations, and Dimensions contained in these plans are calculated from a ecent field survey. The Contractor shall verify all dependent dimensions in the field before ordering or fabricating any material.

All longitudinal and transverse dimensions are measured horizontally and include no correction for arade.

The information shown on these plans concerning the type and location of utilities is not guaranteed to be accurate or all inclusive. The Contractor shall contact the Utility Notification Center of Colorado at 811 (1-800-922-1987) at least 3 days (2 days not including the day of notification) prior to any excavation or other work.

Required chamfering may not be depicted accurately in the drawings. All permanently visible corners shall be formed with $\frac{3}{4}$ " x $\frac{3}{4}$ " chamfer strips unless noted otherwise in the plans. Construction joints at exterior deck/slab/barrier/headwall locations shall be chamfered.

DESIGN DATA

AASHTD, 9th Edition LRFD with current interims

Design Method: Load and Resistance Factor Design

HL-93 (design truck or tandem, and design lane load) CDDT Permit Load (Strength II only) Live Load:

EV3 truck (Strength II only)

Bridge Deck Overlay Dead Load: X psf Permanent Deck Forms

Future Utility Loads

Reinforced Concrete:

4,500 psi 4,500 psi Class D Concrete: Class DF Concrete: f'c Reinforcing Steel: 60,000 psi

Drilled Shaft Concrete:

4,000 psi 60,000 psi Class BZ Concrete: Reinforcing Steel:

Structural Steel:

AASHTO M270 (ASTM A709) Grade 36 AASHTO M270 (ASTM A709) Grade 50 AASHTO M270 (ASTM A709) Grade 70 fy = 50,000 psi fy = 70,000 psiAssumed Single-Lane Design-Life ADTT for fatigue = Bolted Surface Conditions = Class A (slip coefficient 0.33)

Post-Tensioned concrete:

Class S concrete (see details) 270,000 psi

Precast Prestressed Concrete: Class PS concrete (see details)

SEISMIC DESIGN CRITERIA

Earthquake Design method: Force Based or Displacement Based? XX.0000 Latitude =

Longitude =

AASHTO Spectrum for 7% PE in 75 years (1000yr Return Period) Period (sec)

(g) X.000 PGA - Site Class B 0.0 X.000 Ss - Site Class B X.000 S1 - Site Class B 0.2

Spectral Response Accelerations:

As = Fpga*PGA, SDs = Fa*Ss, and SD1 = Fv*S1 Fpga = X.00, Fa = X.00, Fv = X.00 Period (g) X.000 As - Site Class X (sec) 0.0 X.000 SDs - Site Class X X.000 SD1 - Site Class X 0.2

Operational Class:

Seismic Zone or Seismic Design Category: Zone= x or Category= x

Response Modification Factors:

R-Factor: (Substructure type), R-Factor: (Connections)

Soil friction angle, ϕ = XX° (MSE Backfill) Seismic active horizontal pressure coefficient Kae= X.XX Seismic horizontal acceleration coefficient, kh = X.XX *As

INDEX OF DRAWINGS

Change X's to appropriate values. List any other additional

Overlay = 36psf, Deck Forms = 5 psf, Útilities = 5 psf

Do not delete category if not used, change value to 0 psf.

Designer/detailer:

dead loads per Design Manual.

Typical Dead Load values are as follows:

BO1 GENERAL INFORMATION & SUMMARY OF QUANTITIES

Designer/detailer:

I This sheet lists the various general notes I and design data which are commonly ı used. They shallbe modified as necessary I for specific projects.

B-100-1

All bridges shall be designed to carry 36 psf total for both new and future overlay, as applicable. The overlay may be either concrete or asphalt.

Also, place the utility notification graphic on the foundation layout drawing.

ABBREVIATIONS

(Per M-100-2 or as shown below)

Ea BF = Each = Back Face = Front Face FFBW

= Front Face Backwall = Reinforced Concrete WSEL = Water Surface Elevation



BRIDGE DESCRIPTION

Subset Sheets: BXX of XXX

Sheet Number

View/Photo Identification

BXX

Void:

Section, Detail, or View Identification

Cross Reference Drawing Number (if blank or dash, reference is to same sheet)

Sheet Subset:

As Constructed	GE!	Project No./Code			
No Revisions:	SU	Project Number			
Revised:	Designer:	xxxxxxx	Structure	X-XX-XX	Code
	Detailer:	XXXXXXXX	Numbers	X-XX-XX	

BRIDGE

All seals for this set of drawings are applied to the cover page(s)

Print Date: \$DATE\$				Sheet Revisions		
ile Name: Sheet_B-100-1.dgn			Date:	Comments	Init.	
loriz. Scale: None	Vert. Scale: As No	ted				
Jnit Information	Unit Leader In	tials				
				i		

Colorado Department of Transportation

2829 West Howard Place, 3rd Floor Denver, CO 80204 Phone: 303-512-4079 FAX: 303-757-9197

270,000 psi

Staff Bridge Branch Initials