

APPENDIX A
EXAMPLE 1 - ELASTOMERIC LEVELING PAD
METHOD A

GENERAL INFORMATION

Per CDOT Bridge Design Manual (BDM) Section 14.5.7, leveling pads are plain elastomeric pads (PEP) and are designed using Method A procedures in accordance with AASHTO LRFD 7th Edition Section 14.7.6

Leveling pads are primarily used with integral substructures and will not experience shear displacements in that condition. In addition, design for bearing rotation is implicit within Method A procedures (AASHTO C14.7.6.1). The Designer, however, shall confirm that the thickness of the leveling pad is sufficient to prevent girder-to-support contact as a result of anticipated girder rotations, girder skew, and roadway vertical geometry. Leveling pads used with integral substructures are designed for dead loads only, up to and including the deck pour, per BDM Section 14.5.7.

MATERIAL AND SECTION PROPERTIES

Leveling Pad Dimensions

Leveling Pad Width	W = 37.00 in	AASHTO 14.7.5.1
Leveling Pad Length	L = 10.00 in	AASHTO 14.7.5.1
Leveling Pad Thickness	$h_{rl} = h_{rt} = $ 0.75 in	Typically between 1/2" and 1"

Leveling Pad Material Properties

Shore A Durometer Hardness	Duro = 60 (min)	BDM 14.5.7
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Shear Modulus

The least favorable value is assumed since the material is specified by its hardness value (AASHTO 14.7.6.2)

G = 0.13 ksi	AASHTO T14.7.6.2-1
Check = 0.08 ksi < G < 0.25 ksi	OK AASHTO 14.7.6.2

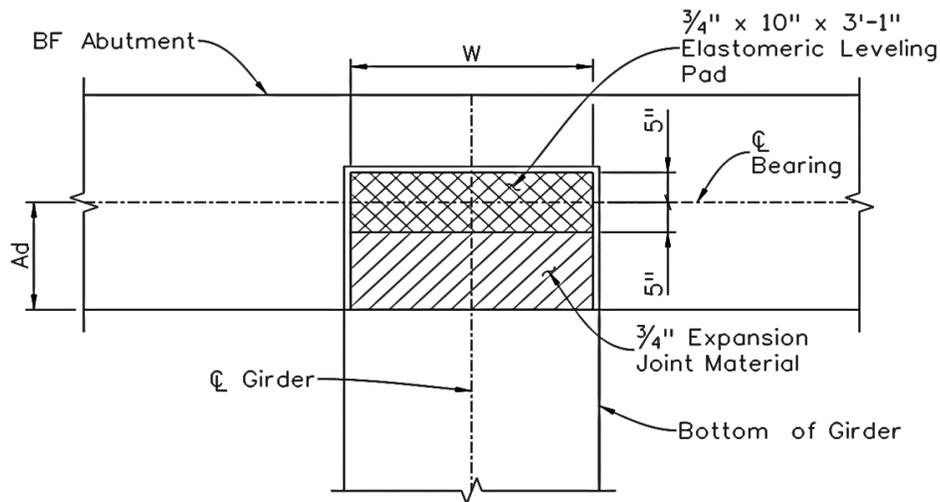


FIGURE 1 - LEVELING PAD DETAIL

BRIDGE GEOMETRY

Profile grade between supports % = **-1.50** %
 C bearing to FF Abutment A_d = **1.25** ft

BEARING ROTATIONS

Rotations include effects of girder camber. For all rotation values, positive indicates an upward rotation while negative indicates a downward rotation.

Service I Limit State Loads

Net girder rotations (camber plus dead loads) θ_d = **0.004** rad

Include a rotational allowance of 0.005 radians due to uncertainties in bearing fabrication and bearing seats. Per BDM 14.5.4, the flatness tolerance for bearing seat uncertainties is accounted for in the rotational allowance.

Construction Tolerance θ_r = **0.005** rad AASHTO 14.4.2.1

BEARING LOADS

Loads acting on the leveling pad are dead load girder reactions, up to and including the deck pour, at the service limit state. Loads are per bearing.

Service I Limit State

DL = **136.00** kip

SOLUTION**Shape Factor**

Total thickness of interior layer, h_{ri} , is equal to total elastomer thickness, h_{rt} ($h_{ri} = h_{rt}$)

Rectangular, plain bearing shape factor without holes:

$$S_i = \frac{LW}{2h_{ri}(L+W)} = \frac{(10.00 \times 37.00)}{[2 \times 0.75 \times (10.00 + 37.00)]} = 5.25 \quad \text{AASHTO 14.7.5.1-1}$$

Compressive Stress

AASHTO 14.7.6.3.2

The compressive stress of the leveling pad shall satisfy the criteria below for a PEP.

σ_s = average compressive stress due to total load from applicable service load combinations

$$\sigma_s \leq 1.00GS_i = 1.00 \times 0.13 \times 5.25 = 0.68 \text{ ksi} \quad \text{AASHTO 14.7.6.3.2-1}$$

and

$$\sigma_s \leq 0.80 \text{ ksi} \quad \text{AASHTO 14.7.6.3.2-2}$$

$$\sigma_s = \frac{DL}{LW} = \frac{136.00}{(10.00 \times 37.00)} = 0.37 \text{ ksi}$$

$$\text{Check } \sigma_s \leq 1.00GS \quad 0.37 \text{ ksi} < 0.68 \text{ ksi} \quad \text{OK}$$

$$\text{Check } \sigma_s \leq 0.80 \text{ ksi} \quad 0.37 \text{ ksi} < 0.80 \text{ ksi} \quad \text{OK}$$

Compressive Deflection

Compressive deflection under initial dead load of a PEP shall meet the following criteria in AASHTO 14.7.6.3.3 and 14.7.5.3.6. Total thickness of interior layer, h_{ri} , is equal to total elastomer thickness, h_{rt} ($h_{ri} = h_{rt}$). Note the graphs presented in Figure C14.7.6.3.3-1 apply to laminated bearings; equation C14.7.5.3.6-1 will be used in lieu of these graphs to determine the strain in the bearing pad under applicable stresses.

$$\delta_d \leq .09h_{ri} = 0.09 \cdot 0.75 = 0.068 \text{ in.} \quad \text{AASHTO 14.7.6.3.3}$$

$$\delta_d = \sum \varepsilon_d h_{ri} \quad \text{AASHTO 14.7.5.3.6-2}$$

$\varepsilon_d =$ dead load compressive strain in elastomeric pad

$$\varepsilon_d = \frac{\sigma_d}{4.8GS_i^2} \quad \text{AASHTO C14.7.5.3.6-1}$$

$$\sigma_d = \frac{DL}{LW} = 0.37 \text{ ksi}$$

$$\varepsilon_d = \frac{\sigma_d}{4.8GS_i^2} = 0.37 / (4.8 \cdot 0.13 \cdot 5.25^2) = 0.021$$

Check

$$\delta_d = \varepsilon_d h_{ri} = 0.021 \cdot 0.75 = 0.016 \text{ in} < 0.068 \text{ in} \quad \text{OK}$$

Stability

AASHTO 14.7.6.3.6

Total thickness of interior layer, h_{ri} , is equal to total elastomer thickness, h_{rt} ($h_{ri} = h_{rt}$).

Total bearing thickness shall not exceed the lesser of the following dimensions:

$$\frac{L}{3} = 10.00 / 3 = 3.33 \text{ in}$$

and

$$\frac{W}{3} = 37.00 / 3 = 12.33 \text{ in}$$

$$\text{Check} \quad h_{rt} = 0.75 \text{ in} < 3.33 \text{ in} \quad \text{OK}$$

Geometry

Confirm that the thickness of the leveling pad is adequate to prevent girder-to-support contact under anticipated girder rotations and roadway geometry. Assume rotations are about the centerline of bearing.

Maximum rotation, including compressive deflection effects, before bottom of girder comes in contact with the top of support:

$$\theta_{max} = \tan^{-1} \frac{(h_{rt} - \delta_d)}{A_d} = -\tan^{-1} [(0.75 - 0.016) / (1.25 \cdot 12)] = -0.0489 \text{ rad}$$

Rotation of girder due to profile grade of bridge between supports in question:

$$\theta_{grade} = \tan^{-1} \frac{\%}{100} = \tan^{-1} (-1.50/100) = -0.0150 \text{ rad}$$

Total girder rotation, including camber, dead loads, allowances for construction and bearing fabrication uncertainties, and roadway geometry.

$$\theta_{total} = \theta_d - \theta_r + \theta_{grade} = 0.0040 - 0.0050 + 0.0150 = -0.0160 \text{ rad}$$

Total rotations through the deck pour need to be less than the maximum rotation:

$$\theta_{max} \geq \theta_{total} \quad -0.0489 \text{ rad} > -0.0160 \text{ rad} \quad \text{OK}$$