# CTE Preliminary Traffic and Revenue Study 

## Colorado Tolling Enterprise <br>  <br> Colorado Department of Transportation



# CTE Preliminary Traffic and Revenue Study 

Prepared for
Colorado Tolling Enterprise


Colorado Department of Transportation

Prepared by


Wilbur Smith Associates
In Association With


HNTB Corporation
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## Chapter

## Introduction

Due to an increasing need to identify new potential sources of transportation funding, the Colorado General Assembly authorized the creation of a Statewide Tolling Enterprise in 2002. This resulted in the formation of the Colorado Tolling Enterprise (CTE), which is now considering a number of potential candidate projects throughout the state as possible toll facilities. While some relatively new toll facilities already exist, primarily in the Denver area, an expanded use of the toll concept is being considered, primarily the "new capacity" added to the highway system. Following its creation, CTE initiated a process of identifying potential toll projects for consideration. At the outset, over 90 candidate projects were considered by CTE, and subjected to a very "broad-brush" review process.

Figure 1-1 provides a graphic representation of the tolling evaluation and study process envisioned by CTE. It is a multi-phase process, with each subsequent step adding an increased level of analytical detail. The process eliminates some candidate projects at each phase, culminating in a reduced number of projects being subjected to progressively more detailed analyses. The initial 90 -plus candidate projects were subjected to an initial screening process by CTE based on "broad-brush" evaluation criteria, including:

- Volume/capacity ratios of 0.7 or more, as a measure of relative congestion levels (considered at both 2001 and 2030 levels);
- Average daily traffic volumes in excess of 30,000 vehicles per day (considered at both 2001 and 2030 levels);
- Average daily truck volumes in excess of 1,500 per day (considered at both 2001 and 2030 levels);
- Roadway classification (such as freeway or expressway) as it may effect opportunities for tolling and moving higher volumes of traffic at higher speeds;
- Projected population growth of 100 percent or more between 2000 and 2025;



## COLORADO TOLL CANDIDATE SCREENING AND STUDY PROCESS <br> FIGURE 1-1

As a result of this screening process, more than half of the projects on the original candidate list were considered "low priority" and were essentially eliminated from further consideration. Approximately 40 of the projects were considered to have "high" or "medium" potential, meriting further consideration in subsequent, more detailed analyses.

## TOLL CANDIDATE SCREENING PROCESS AND STUDY PURPOSE

A study team lead by Wilbur Smith Associates (WSA), and including HNTB Corporation (HNTB), Felsburg Holt \& Ullevig (FHU), and

Wilbur Smith Associates

Citigroup, was selected by CTE to perform the more detailed and refined traffic and revenue analyses envisioned in Phases I and II of the study process depicted in Figure 1-1. As shown, in Phase I of the overall study, the "first-tier" of the screening process was undertaken, starting with the "high" priority projects and a portion of the "medium" priority projects as identified in the initial CTE screening analysis. This required a new set of screening criteria to be developed by the study team and submitted for approval by CTE.

This first-tier screening was still a generally subjective analytical approach, albeit somewhat more detailed and rigorous than the initial screening process performed previously by CDOT. In performing the first-tier analysis, maximum benefit was derived from the experience of the WSA-led study team, having previously analyzed hundreds of facilities across the nation. A summary of the first-tier screening process and findings are presented in Chapter 2 of this report.

The candidate projects surviving the first-tier screening process were then subjected to a more detailed, although still preliminary, second-tier feasibility analysis and are the subject of this report. Wherever possible, available travel demand models were used to develop preliminary estimates of traffic and revenue potential, optimum toll levels and revenue growth potential in the second-tier analyses. In parallel, the study team also refined project capital, and maintenance and operating cost estimates initially developed during the first-tier screening process. Together with the estimates of toll revenue, capital, and maintenance and operating costs, a financial feasibility assessment was performed in the Phase 2 , secondtier analysis.

## SCOPE OF WORK

This study was intended to provide the CTE with a preliminary feasibility analysis on the list of second-tier candidate toll projects. In order to complete this analysis, a number of major work tasks were performed and are described below.

## Coordination With On-Going Nepa Studies

There are several corridor studies now underway, largely in and around the Denver area. These include projects which may have toll potential, and in some cases have already been proposed by the private sector as new capacity toll projects. Because of this, it was important to coordinate closely with these on-going studies. Representatives of the study team participated in corridor coordination meetings, as required, and provided

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input on tolling issues at appropriate points during the study process. The team also obtained and used any available data from these studies for input into this preliminary feasibility study.

## Model Development

Second-tier candidate toll projects were subjected to more detailed, but still preliminary, traffic and revenue analyses. These analyses made use of travel demand models to make traffic assignments at opening and future year levels, and at alternative toll rates.

At the outset of this task, WSA obtained the latest versions of all available regional travel demand models, including:

- Denver Regional Council of Governments (DRCOG);
- Pikes Peak Area COG (Colorado Springs area);
- North Front Range MPO (Ft. Collins area); and
- I-70 West Mountain Corridor model.

In addition to networks themselves, WSA obtained underlying socioeconomic forecasts at the traffic zone level at all available years. Wherever possible, trip tables for peak and off-peak conditions were obtained. This was important since many of the candidate toll projects involved placing tolls only on "new capacity" on existing toll-free routes.

As required, the specific projects were "recoded" into the respective travel demand models to permit use with WSA's toll diversion algorithms within its traffic assignment software. This software was developed specifically to estimate the market share of total traffic demand willing to pay tolls for different toll project configurations at different price levels, compared with the best alternative toll-free routing. Hence, project coding was critical to the simulation process.

Since many of the projects would involve the tolling of new capacity only, it was important to disaggregate trip tables to reflect various time periods of the day. In general, this included a.m. peak, a.m. shoulder, midday, p.m. shoulder and p.m. peak conditions. When only the new capacity is priced and toll-free capacity remains available in immediately adjacent lanes, hourly and directional distributions of traffic are important factors in determining the share of traffic willing to use the current lanes. These types of managed-lane facilities typically have higher utilization during peak periods and very low utilization in off-peak hours which require the use of tolls which vary by time of day.


For purposes of this second-tier preliminary analysis, traffic assignments were made for each of the managed-lane projects by principal time period of the day. Alternative toll rates were also tested to determine the sensitive equilibrium point between the toll-free and tolled lanes, on these types of projects. For potential toll candidate projects which are more traditional in nature, such as a stand-alone new toll facility, the same basic, but less complex traffic and toll assignment process was followed.

## Traffic and Revenue Analysis

For each second-tier project, a series of traffic assignments using the models described above were performed. Traffic assignments were made at opening (2010) and future (2025 or 2030) years. At the outset of the study, it was recognized that most of the projects analyzed would not have advanced through the planning and design phases at a pace fast enough to allow them to open to traffic by the year 2010. However, this year was chosen to allow all projects to have "equal footing," being analyzed over the same time horizon. Additionally, WSA, as requested by CTE, can reassess the traffic and revenue potential of projects, when DRCOG updated 2030 models become available. Intermediate year revenue forecasts were developed through interpolation. Similar forecasts beyond 2025 were estimated through extrapolation. Thirty-year toll revenue projections were prepared for each project alternative; nominal assumed traffic growth rates were used to extend revenues beyond the horizon year of the assignment process.

At opening and horizon year levels, a range of alternative toll rates for each project were tested. This was particularly critical for projects where only the "new capacity" was being priced. In such cases, it was important to identify optimum toll rates for peak, shoulder and off-peak conditions. Traffic assignment results were then reviewed for reasonableness.

For each of the projects tested, daily traffic estimates, as well as peak hour traffic estimates where needed, were prepared. Annual revenue estimates were developed for each project, extending over a 30 -year projection period.

## Corridor Development Cost Estimates

This task entailed identifying the improvement characteristics and associated construction and maintenance costs for each candidate toll project to a higher degree of detail than in the first-tier screening. The cost estimate methodologies involved updating and refining the available information from CDOT to establish typical improvement standards and construction cost build-up tables for the various facility types. These standards were then applied to the various corridors based on the
definition of each corridor's improvements. The definition of the necessary improvements to each corridor depended on the current configuration of the existing roadway, if applicable, and the nature and extent of the facility upgrades. The associated construction and on-going maintenance cost estimates were based on the application of the typical standards to the identified improvements to each corridor.
Utilizing available bid tab information and cost estimates from earlier corridor studies, cost build-up tables were developed to estimate the construction costs. Overall unit-cost factors were developed from the costbuild up tables at a greater level of detail than in the first tier screening process. These unit-cost factors included items such as paving, grading, and drainage. The unit-cost factors for each improvement type represented typical applications and were adjusted as required for special considerations such as major bridge crossings and interchanges. Appropriate add-ons for "soft" costs associated with design, right-of-way acquisition, and program management and administration were also considered to develop a total capital roadway cost for implementation.

The following items were included:

- Paving, Shoulder and Base;
- Grading;
- Drainage;
- Utility Relocations;
- Lighting, signing and pavement markings;
- Erosion control;
- Interchanges (excluding bridges);
- Bridges/Structures;
- Construction incidentals and miscellaneous items;
- Right-of-way; and
- Project Contingency

Also included in the capital cost estimate for each project is the cost for electronic toll collection (ETC) equipment and installation. The unit costs for ETC equipment and installation were based on recent bid tabulations from other comparable turnpikes and other toll facilities operating in Colorado, as well as previous team experience on other toll projects.

It should be noted that cost estimates prepared for this analysis were based on bid tab data and cost estimates from either prior studies, or in the case of on-going environmental studies, from then-current project cost estimates provided by the corridor study teams. As these project configurations are refined over time, it is highly probable that the cost

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estimates will also be refined and may differ from those presented in this report.

## Financial Feasibility Analysis

Following the development of traffic and revenue forecasts, the study team brought the various analytical results together into an analysis of the financial feasibility of the potential second-tier toll candidate projects. The study team then undertook a financial feasibility assessment, using a discounted cash flow model.

Each potential toll project was evaluated for financial feasibility based upon these initial construction estimates and the traffic and revenue forecasts developed by the study team, and then applying that data to a discounted cash flow analysis. This analysis determined the capacity of the proposed toll project to support debt. The analysis also included setting aside sufficient reserves for unplanned major maintenance or construction, for debt service, and for rate/toll stabilization.

Each project was analyzed as a stand-alone, single asset facility and then, several select projects were analyzed under an integrated system approach to gauge levels of feasibility.

## ORDER OF PRESENTATION

The report has been ordered based on the following chapter structure:
Chapter 2 provides an overview and summary of findings for the first-tier screening process. Specifically identified are the initial list of all candidate toll projects, screening objectives and methodology, and the screening evaluation criteria. The list of candidate projects to be evaluated in the second-tier screening process is identified.

Chapter 3 presents preliminary traffic and revenue estimates for the Denver area candidate toll projects included in the second-tier analysis, while Chapter 4 presents similar findings for the Colorado Springs, I-70 Mountain Corridor, and other statewide candidate toll projects. The first portion of these chapters provides a description of the second-tier screening approach and analysis methodologies for traffic modeling and toll collection system design. Following this, a description of each toll project is provided, along with discussions of existing traffic conditions, access locations and tolling concept, and estimates of traffic and annual toll revenue.


Chapter 5 presents preliminary estimates of capital, operating and maintenance costs for each project. Included are those costs associated with both the roadway and the toll collection system. These costs are based on bid tab data and cost estimates from prior studies, or in the case of on-going environmental studies, from current cost estimates provided by these corridor study teams. These costs will likely change as the project concepts are refined. The chapter provides an overview of the general methodology and basic assumptions used in preparing the estimates. Also provided are reserve fund deposit cost estimates, those costs associated with setting aside annual amounts for unplanned major maintenance or reconstruction activities.

Chapter 6 presents the results of the financial feasibility analysis including the methodology and financing assumptions and the feasibility of each project as a stand-alone toll facility. Also included is a feasibility analysis of several projects combined into an integrated system approach.

Finally, Chapter 7 presents a concise set of suggested next steps toward possible implementation of a toll facility system in the state.

## Сhapter

## 2

## First-Tier Screening Process And <br> Findings

Prior to the commencement of WSA's first-tier screening study, the Colorado Tolling Enterprise (CTE) conducted a preliminary evaluation of potential candidate toll facility projects in Colorado. Through its own broad screening approach, 39 candidate projects were selected out of more than 75 potential pojects. These 39 projects, in various configurations, were evaluated by WSA in a first-tier screening, intended to facilitate the selection of projects to be studied in the second-tier phase of evaluation.

The findings of the first-tier evaluation phase resulted from application of 12 first-tier screening criteria developed in "Technical Memorandum No. 1 - Proposed First-Tier Screening Criteria," as well as consideration of public comments. Of necessity, the analytical approach used was largely subjective in nature, making maximum use of available information, such as traffic counts, historical construction costs, information from prior studies, and professional judgments. At this level of study, it was not appropriate to conduct a detailed traffic or engineering analysis of each of the corridors; rather, each project was analyzed using a "broad-brush" approach, with care taken to ensure consistent levels of analysis between projects, to the maximum extent possible.

Twelve "first tier" screening criteria were used, as identified in the aforementioned "Technical Memorandum No.1." These include, in no particular order of importance:

- Potential Safety Impacts;
- Toll Operations Viability Assessment;
- Economic Growth Considerations;
- Consistency with Statewide and Regional Plan Goals;
- Community Impact Assessment;
- Congestion Relief Potential;
- Network Continuity Considerations;
- Order-of-Magnitude Construction Cost Estimates;


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- General Constructability Assessment;
- $20^{\text {th }}$ Year Traffic and Revenue Potential;
- Relative Financial Feasibility Index; and
- Other considerations.

Detailed descriptions of these criteria can be found in the previously submitted technical memorandum, "Proposed First-Tier Screening Criteria, Candidate CTE Toll Facility Project."

## CANDIDATE PROJECT LIST OVERVIEW

Table 2-1 presents a list of all 39 projects evaluated in this screening. Indication of the type of each project is also given, using the following categories: (1) managed lanes, (2) new toll roads, (3) managed facilities (new limited-access lanes constructed in the right-of-way of an arterial roadway), (4) truck toll lanes, (5) toll tunnels, and (6) conversion of high occupancy vehicle (HOV) lanes to high occupancy toll (HOT) lanes. The distinction between these project types is as follows:

- Managed Lanes - These projects typically involve the addition of one or two new lanes per direction along existing freeways, where only those drivers using the new capacity are required to pay a toll.
- New Toll Roads - These involve new construction (either within or adjacent to existing roadways) for which all vehicles using any portion of the new roadway would be assessed a toll. In general, competing routes for the new toll roads would be existing two-lane or multi-lane state highways, with any existing traffic signals, lower posted speed limits, and sections passing through various towns and cities.
- Managed Facilities - This type of project would generally involve construction of new toll facilities, possibly as elevated roadways, along existing arterial routes. While these would be similar to managed lanes, the immediately competing lanes would usually be signalized and have lower operating speeds, whereas "managed lane"type projects would involve adding new capacity to existing freeways where the competing lanes would operate under comparable speed limits without signalized intersections.

Table 2-1
First-Tier Screening Projects

| No. | Type | Type Description | Roadway | Project Limits |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 1 | Managed Lanes | I-25 | I-70 to Fort Collins |
| 2 | 1 | Managed Lanes | I-70 | C-470 to I-25 |
| 3 | 1 | Managed Lanes | I-70 | I-25 to E-470 |
| 4 | 1 | Managed Lanes | I-25 | C-470 to Colorado Springs |
| 5 | 1,5 | Managed Lanes, Tunnel | I-70 | Eagle to C-470 |
| 6 | 6 | HOT Lanes | U.S. 36 | I-25 to Boulder |
| 7 | 3,6 | Managed Facility, HOT Lanes | U.S. 85 | I-25 to C-470 |
| 8 | 1 | Managed Lanes | C-470 | I-70 to I-25 |
| 9 | 1 | Managed Lanes | I-25 | Colorado Springs to Pueblo |
| 10 | 1 | Managed Lanes | I-76 | I-70 to E-470 |
| 11 | 1 | Managed Lanes | 6th Avenue | C-470 to I-25 |
| 12 | 3 | Managed Facility | U.S. 85 | I-76 to U.S. 34 |
| 13 | 1 | Managed Lanes | I-70 | Utah to Eagle |
| 14 | 1 | Managed Lanes | I-225 | S.H. 83 to I-70 |
| 15 | 3 | Managed Facility | U.S. 40 | C-470 to I-25 |
| 16 | 4 | Truck Only Lanes | I-76 | E-470 to Nebraska |
| 17 | 2 | New Toll Road | U.S. 24 | I-25 to Limon (I-70) |
| 18 | 3 | Managed Facility | U.S. 24 | S.H. 67 to I-25 |
| 19 | 1 | Managed Lanes | I-25 | Fort Collins to Wyoming State Line |
| 20 | 3 | Managed Facility | U.S. 285 | Conifer to U.S. 85 |
| 21 | 2 | New Toll Road | 70 Business | SH 340 to I-70 |
| 22 | 3 | Managed Facility | U.S. 34 | I-25 to S.H. 85 |
| 23 | 6 | HOT Lanes | S.H. 82 | Glenwood Springs to Aspen |
| 24 | 3 | Managed Facility | U.S. 85 | C-470 to I-25 |
| 25 | 4 | Truck Only Lanes | I-70 | E-470 to Kansas State Line |
| 26 | 3 | Managed Facility | S.H. 83 | I-225 to E-470 |
| 27 | 3 | Managed Facility | S.H. 119 | Boulder to I-25 |
| 28 | 4 | Truck Only Lanes | U.S. 287 Bypass | I-25 to Livermore |
| 29 | 2 | New Toll Road | Powers Boulevard | I-25 North to I-25 South |
| 30 | 3 | Managed Facility | S.H. 121 | U.S. 36 to C-470 |
| 31 | 3 | Managed Facility | S.H. 391 | I-70 to U.S. 285 |
| 32 | 2 | New Toll Road | U.S. 50 | I-25 (Pueblo) to Kansas State Line |
| 33 | 1 | Managed Lanes | S.H. 58 | S.H. 93(Golden) to I-70 |
| 34 | 2 | New Toll Road | NW Corridor | U.S. 6 to NW Parkway |
| 35 | 2 | New Toll Road | S.H. 9 | I-70 to U.S. 40 |
| 36 | 3 | Managed Facility | S.H. 9 | I-70 to Breckenridge |
| 37 | 2 | New Toll Road | Front Range | Fort Collins to Pueblo |
| 38 | 2 | New Toll Road | Banning-Lewis Parkway | Colorado Springs from I-25 N. to I-25 S. |
| 39 | 1 | Managed Lanes | I-270 | US 36 to I-70 |

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- Truck Toll Roads or Lanes - These projects would involve either adding a lane in each direction along interstate routes with heavy truck volumes, or constructing new truck lanes (or truck toll roads) to relieve existing arterial routes which currently experience heavy truck volumes. In both cases, use of the new facilities or lanes would be limited to trucks.
- Tunnels - These projects (the Eisenhower Tunnel and the Idaho Springs Tunnel on I-70) would involve constructing an additional "bore" to meet the peak traffic demands characteristic of the I-70 corridor.
- HOT Lanes - Although essentially identical to managed lanes in physical configuration (i.e. the addition of a tolled lane or two in each direction on an existing freeway), these facilities would offer toll-free access to high-occupancy vehicles. Only single-occupant vehicles would be assessed a toll for use of the new lanes.

Different methodological approaches were used for each of these project types when assessing viability with respect to the aforementioned screening criteria. The particular processes used and factors considered are explained in further detail below, as well as in the previously submitted Technical Memorandum, "First-Tier Screening Process and Findings."

## SCREENING METHODOLOGY AND EVALUATION CRITERIA

Presented below is a brief overview describing each of the criteria used to evaluate the 39 candidate toll projects selected as part of the first-tier screening analysis. A more in-depth discussion of these criteria including, the specific data gathered for each criteria and the data sources and application to specific criteria can be found in "Technical Memorandum No. 1 - Proposed First-Tier Screening Criteria," as well as "Technical Memorandum No. 2: First-Tier Screening Process and Findings," both of which have previously been submitted to the Colorado Tolling Enterprise.

Safety Impact - Data on the location and type of accidents on major Colorado highways in 2001 were provided to WSA by CDOT. These were converted into weighted average accident rates per million vehicle miles of travel (VMT) for highway segments within project corridors, and compared to statewide weighted average accident rates for facilities of the same type. Facilities with accident rates substantially greater than the statewide average for the appropriate facility type were determined to have

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relatively high potential for safety improvement, and the projects located within the associated corridors were given higher safety impact ratings.

Toll Operations Viability Assessment - Toll operations viability ratings were determined by taking into account a particular set of requirements and considerations for any given proposed facility according to its project type. To the extent that a given project could be constructed in such a manner as to fully satisfy all of the specified tolling system requirements, it was assigned a high toll operations viability rating. Projects with critical or fatal barriers to satisfaction of one or more of these requirements would conversely be assigned lower toll operations viability ratings.

Economic Impacts - A project was determined to have positive economic impacts if it would support an area where significant growth was expected or if it would provide access to areas with growth potential not currently well served by the existing highway network. To identify such projects, WSA defined an influence area for each proposed toll facility, examined current and projected socioeconomic conditions for these areas, and determined the extent to which each proposed toll facility could catalyze growth (growth considerations) in areas currently stifled by poor accessibility.

Consistency with Statewide and Regional Plan Goals - Projects were reviewed to assess consistency with other on-going statewide and regional transportation plans. The documents consulted included Corridor Visions being prepared in the 2030 Statewide Transportation Plan, the 2003 list of Strategic projects, select Transportation Improvement Programs (TIPs), and other comments received by the Consultant team. Broad categories of consistency were defined as follows:

- The proposed project or similar improvement is on the plan or the project provides needed congestion relief or improves freight flow.
- The proposed project is not on the plan as such, however, it is consistent with the spirit and goals enumerated for the plan or is not a highly consistent project but has been the subject of a recent EIS.
- The project is not on the plan and not consistent with the spirit of the plan.

The Denver Regional Council of Governments also provided information regarding their future plans and their assessment of the extent to which envisioned projects in their area may not be consistent with future plans.

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Community Impact Assessment - This evaluation criterion is a "broad brush" relative measure of the social and environmental impacts that the tolling improvements on a corridor would have on adjoining land uses and communities. Several key factors were considered in this category of impact, including:

- Social Impacts - A general assessment of the need for additional right-of-way and the displacement of residences or businesses along the corridor;
- Aesthetic Impacts - The vertical profile (at-grade versus elevated) and any uniquely sensitive topography through which the project would be constructed;
- Environmental Justice Issues - The potential for disproportionately high and adverse effects on low-income or minority populations;
- Natural Environment Impacts - Assessed on the basis of development of the corridor. If the improvements create a new transportation corridor or expand a corridor through an undeveloped area, it is more likely to have a significant impact on the natural environment. The presence of a unique feature of the natural environment increases the potential for a significant impact; and
- Noise and Air Quality Impacts - Noise assessment is directly related to the density and proximity of development in the corridor, while the assessment of air quality impacts is primarily related to the congestion relief provided by the proposed improvements.

It should be emphasized that in the first-tier screening process, the assessment was qualitative in nature and was not based on detailed quantitative data. It was based largely on the characteristics of the corridor that were either known or could be observed in the field.

Congestion Relief Potential - To assess relative congestion relief potential, each project was divided into segments, delimited by groups of uniform roadway attributes as given in CDOT's geographic information systems (GIS) database. The level of congestion relief for each project was defined as low, medium, or high based upon the percentage of VMT exposed to highly congested roadway segments. For example, toll projects related to a facility with a 2020 VMT of 33 percent or less over the V/C threshold of 0.85 would be expected to provide a low level of congestion relief; projects with 34 to 65 percent would be expected to

provide a medium level of congestion relief; and over 65 percent would be expected to provide a high level of congestion relief.

Network Continuity Considerations - Each project was evaluated in terms of its relationship to other toll facility projects, with regard to connectivity and the development of a coherent toll network. For example, a particular project could enhance the performance and viability of a nearby project. A general review was made of each of the facilities to determine the extent to which they would function as an integral element of an overall improved network, or operate in isolation.

Order-of-Magnitude Construction Cost Estimates - The purpose of this evaluation criterion was to identify, given the type of improvements planned in each corridor, order-of-magnitude construction costs for the corridors in question. The construction costs for each project were then compared with a relative measure of the project's projected toll revenue to develop a Relative Feasibility Index.

Constructability Rating - The purpose of this evaluation criterion was to identify a relative measure of the ease or difficulty of constructing a project (apart from cost considerations). The primary elements that would influence the constructability of the corridors involve construction conflicts with existing roadways or development, constraints due to topographic features, and inadequate subsurface soils. The ease of construction of each corridor would be evaluated using a relative rating system. Each corridor would be given a low, medium or high impact rating for constructability, where a high impact rating indicates that there would be higher constructability issues to implement the project. The constructability evaluation would identify characteristics associated with each corridor that may make construction of the corridor difficult or infeasible.

20th Year Traffic and Revenue Potential - An estimate of 20th year traffic and revenue potential was prepared for each of the first tier projects. A generalized assessment was made to estimate traffic demand via generalized toll diversion and toll sensitivity for each of the project corridors using manual traffic assignment and traffic estimation techniques. Regional travel demand models were not used in this phase of study. Instead, maximum use was made of existing datasets and prior studies where available.

For each of the projects a comprehensive set of data was gathered for every individual highway segment using datasets provided by the CDOT. These included but were not limited to existing and 20th year annual

average daily traffic, historical traffic counts, hourly traffic profiles, daily and seasonal variations, truck percentages, posted speed limits, functional classifications, number of lanes, and lane widths. The projects were then analyzed using "spreadsheet" models and methodologies appropriate to each proposed improvement.

Financial Feasibility Index - While a more detailed financial feasibility analysis has been undertaken in the second-tier phase of study, the firsttier screening assessment included an attempt to provide a relative index (for comparison between projects) of financial viability. The index, shown in Table 2-2, is a simple and direct comparison between $20^{\text {th }}$ year revenue potential and estimated capital cost. The intent is to provide a relative indication of feasibility, and not to determine if any individual project is actually financially viable. Lower values of this index are given to more viable projects. In fact, this number may be thought of as a general indicator of the number of years required for a project to pay back its construction costs from the toll revenues received.

Table 2-2
Relative Financial Feasibility Index

| Relative Financial <br> Feasibility Index | Ranking <br> Factor |
| :---: | :---: |
| $>50$ | Very low |
| $40-50$ | Low |
| $30-40$ | Medium-low |
| $20-30$ | Medium |
| $10-20$ | Medium-high |
| $<10$ | High |

## SUMMARY OF FINDINGS

Table 2-3 presents the final list of project corridors advancing to the second-tier analysis. A total of 12 project corridors are shown, some of which have multiple alternatives. The detailed results of this second-tier analysis are given in the following chapters. While somewhat more detailed than the broad-brush screening analysis documented in this chapter, this analysis is still preliminary in nature. Considerably more detailed studies would be needed, beyond the second-tier analysis, before any of these projects could proceed to actual financing.

Table 2-3
Final Tier 2 Candidate Toll Facilities

| Project <br> Number | Roadway |  |  |
| :---: | :---: | :---: | :---: |
|  |  | I-25 | Project Limits |
| 3 | I-70 | I-70 to Fort Collins |  |
| 5 | I-70 | I-25 to E-470 |  |
| 6 | U.S. 36 | Idaho Springs/Eisenhower Tunnels |  |
| 8 | C-470 | I-25 to Boulder |  |
| 14 | I-225 | I-70 to I-25 |  |
| 28 | U.S. 287 | S.H. 83 to I-70 |  |
| 29 | Powers Boulevard | I-25 to Livermore |  |
| 38 | Banning-Lewis Parkway | Colorado Springs from I-25 N. to I-25 S. |  |
| 34 | NW Corridor | U.S. 6 to NW Parkway |  |
| 37 | Front Range | Fort Collins to Pueblo |  |
| 39 | I-270 | I-70 to U.S. 36 |  |
|  |  |  |  |

## Сhapter

## Preliminary Estimates Of Traffic And

 Toll Revenue Denver Area Candidate ProjectsPresented in this chapter are the estimates of traffic and toll revenue for the second-tier toll candidate projects in the Denver area. In total, there are 14 project alternatives in 7 general highway corridors. These project corridors are depicted in Figure 3-1. In addition to the traffic and revenue estimates, existing daily traffic volume data, where available, are discussed, along with the assumed project access points and tolling concepts. Tables showing the estimated optimum toll rates by time period and travel direction are also provided.

## SECOND-TIER SCREENING STUDY APPROACH

As discussed in Chapter 2, the first-tier screening process was conducted on a larger set of candidate toll projects and was based largely on a subjective assessment of traffic and revenue potential, and project cost, using readily available information and a more simplified analytical approach. This second-tier analysis has been conducted on a reduced number of project corridors and project scenarios, but made use of the travel demand models of the Denver Region Council of Governments (DRCOG) in developing traffic and revenue estimates. In addition, as will be discussed subsequently, a more detailed analytical approach was also used in developing preliminary estimates of capital, operating and maintenance costs for each candidate toll project. The second-tier analysis also brought together these estimates of revenue and cost to evaluate the financial feasibility of each project as will be described in Chapter 6.

## PROJECT OVERVIEW

Presented in Table 3-1 is a list of the 14 second-tier candidate toll projects in the Denver area. The table provides the project location, limits and a
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brief description of the type of toll facility, either express toll lanes, of which there are 12 analyzed, or new toll roads of which there are two. More detailed descriptions of each project are provided below.

## ANALYSIS METHODOLOGY

Presented below is a brief discussion of the methodology used to prepare the traffic estimates for each of the Denver area projects, along with a similar discussion of the methodology and considerations taken into account in developing the toll collection system for each project.

## Traffic Modeling

Twelve of the fourteen project scenarios studied within the Denver area are "managed lane" type facilities. In these projects, existing freeways are widened, with tolls charged only on the added new lanes. Existing capacity generally remains toll-free. Toll charges in the managed lanes vary based on demand, to ensure lanes continue to flow freely.

The traffic and revenue estimation process for the managed lanes projects was a multi-step process that incorporated actual traffic counts, travel time information collected from travel time runs, the regional travel demand model, and a micro-model of the corridors. Major work elements of this forecasting process included the following:

- Develop an existing traffic operations profile in each corridor;
- Develop a micro-model of each corridor with estimates of opening and future year global traffic demand;
- Estimate market share under tolled conditions; and
- Estimate annual revenue.

The remaining two projects, the Northwest Corridor Toll Road, Scenarios 1 and 2, used the Denver regional travel demand model for estimation of traffic and revenue. Toll sensitivity analyses were performed at opening and future year levels. Selection of optimum toll rates were made, and traffic and revenue was summarized for each of these two projects on a daily basis.

The most recent version of the travel demand model produced by the DRCOG was used as a starting point in the analytical process. This newer version is based on a TRANSCAD modeling framework, but utilizes socioeconomic data forecasts by traffic zone which had been developed in the previous version of the model. DRCOG is developing an updated future year 2030 version of its TRANSCAD model. Therefore, traffic and

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revenue estimates presented in this report may be revised in the future based on this updated model through additional analyses, if requested by CTE.

At the outset of the work, discussions were held with DRCOG staff regarding basic network assumptions made when developing travel demand matrices (trip tables), which provide the underlying basis for estimating demand for travel in the region. DRCOG staff indicated that beyond currently committed major capacity expansions, now under construction, primarily along I-25 and any portion of I-225, future capacity expansions on other freeways, such as the various managed lane proposals addressed as part of this study, were not assumed in the distribution process. This would tend to underestimate growth and demand in some of the major freeway corridors. As a result, and with assistance from DRCOG staff, WSA used the TRANSCAD framework to develop updated trip tables, using the same socioeconomic forecasts but reflecting higher nominal capacities along the freeways in which managed lane projects were to be evaluated.

In most cases, this resulted in slightly higher estimates of "global" demand in each of these freeway corridors, than represented by the original DRCOG trip tables. The differences, however, were not that significant, generally between 5 and 15 percent of the baseline demand, by the year 2010.

In addition, the version of the model provided by DRCOG did not include the future "FasTracks" regional rail transit initiative, which was passed as a referendum item in the November elections. However, at the time the information was provided by DRCOG, there was uncertainty about whether the future transit initiatives or initiative program would be approved, and therefore decisions were made to use modeling inputs which did not include the major future new rail initiatives, although other previously committed new transit services were included in the network and distribution process.

Three of the FasTrack planned corridors could potentially affect demand in one or more of the proposed managed lane facilities studied as part of this analysis. In theory, construction of competing rail service in the immediate corridor with managed lanes would tend to reduce the demand for the toll facility; although, the extent of this would need to be evaluated in much more detailed studies. Since this study was not intended to be performed for use in direct support of possible project financing, it was determined to be most reasonable to assume that the program was not in place for purposes of this feasibility study. However, any possible

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"investment grade" traffic and revenue studies for any of these corridors would, of course, need to more closely examine the impacts of the approved proposed FasTrack program.

Develop Corridor Pricing Micro-Models - Consistent with the preliminary nature of this study, the analysis was performed over a limited time period, and was based primarily on readily available information. Available traffic counts along the study corridors were obtained from previous studies conducted by WSA and other consultants. In addition, WSA staff conducted field reconnaissance to determine roadway characteristics, roadside constraints, and typical travel time and operating conditions during peak and off-peak periods.

A balanced hourly traffic profile along each of the managed lane project corridors was created by using mainline and ramp hourly traffic count data. Based on these hourly variation patterns and the travel time runs, the 24-hours of the day were divided into peak, shoulder, and off-peak time periods. The following represents the time intervals for which the analyses were performed:

- AM "Pre-Shoulder" - 6:00-7:00 a.m.
- AM Peak - 7:00-8:00 a.m.
- AM "Post Shoulder" - 8:00-9:00 a.m.
- Mid Day - 9:00 a.m. - 3:00 p.m.
- PM "Pre-Shoulder" - 3:00-4:00 p.m.
- PM Peak - 4:00-6:00 p.m.
- PM" Post Shoulder" - 6:00-7:00 p.m.

DRCOG regional travel demand model was used to develop the background travel patterns in the corridors, estimate potential diversion from other parallel streets and highways due to the added available capacity, and to identify potential future growth in the corridor. The managed lanes as well as the access/egress points were coded into the regional travel demand model reflecting the current configuration of the managed lane facilities.

The corridor pricing micro-model was extracted as a window from the regional model. Potential diversions to each of the project corridors from other roads were estimated using the regional model by allowing traffic into the managed lanes. The base regional model trip tables were disaggregated and adjusted to represent the analysis periods listed above. These disaggregated trip tables were then adjusted using a matrix adjustment process to match the balanced hourly traffic volumes on the ramps and mainline segments of each study corridor.

Future model runs of the regional travel demand model for the corridor were performed at year 2025 levels to provide growth adjustments to the calibrated base year trip tables by the period.

Optimum Toll Rate Analysis - Traffic and revenue analysis is based on an estimate of the amount of traffic willing to pay a toll of \$X to save Y minutes. The pricing micro-model attempts to find the equilibrium point between the amount of time savings and willingness to pay the prevailing toll rate.

Within the model, for each origin-destination pair, the travel time using the managed lanes is compared to the travel time using a toll-free routing (on the freeway or its adjacent streets) to estimate a travel time savings. The toll charged for each movement is compared to its time savings to estimate a ratio of "cost-per-minute-saved." This cost-per-minute-saved is compared to the value-of-time for travelers. Those travelers with values-of-time higher than the cost-per-minute saved would tend to choose the tolled lanes, while those with lower values-of-time would tend to choose the general purpose lanes.

Drivers' values-of-time are not uniform, so for any given toll rate/time savings combination, only a portion of those eligible to use the managed lanes would actually choose to use them. As traffic moves from the general purpose lanes to the managed lanes, the time savings advantage offered by the managed lanes is reduced. For each toll rate level, the pricing model finds the equilibrium point between changes in travel time due to traffic shifting to the managed lanes and willingness-to-pay based on value-of-time and travel time savings.

Traffic analyses were performed for years 2010 and 2025. Toll sensitivity tests were conducted for each project, for each time period/direction by running a full range of toll rates from $\$ 0.05$ to $\$ 0.50$ per mile and summarizing traffic and revenue for each rate level. Toll rate selection for each time period/direction was based on a combination of criteria, including maximizing revenues. The optimum toll rates selected are discussed below for each candidate toll project.

## TOLL COLLECTION SYSTEM CONSIDERATIONS

Toll facilities within the metropolitan area are all assumed to be express toll lanes that supplement existing general purpose lanes except for open road tolling solutions for the unfinished portion of the existing Northwest Corridor. Open road tolling involves the exclusive use of express toll
lanes that record either a toll or violation transaction at the prevailing highway speed in non-stop, continuous highway lanes. Tolling is typically accomplished electronically by reading an encoded transponder attached to the vehicle's windshield by an overhead antenna. The inherent assumption when implementing open road tolling is electronic toll collection (ETC) will initially support customer participation to generate sufficient revenue. The CTE projects directly benefit from the identical ETC technology used by both the E-470 Public Highway Authority and the Northwest Parkway Authority.

Another important system implementation consideration is accommodation of either "pre-set" time of day (TOD) or "dynamic" variable pricing to more accurately capture the increasing value of the differential time savings realized during periods of congested flow. Pricing can be transactional based, whereby a user charge is recorded for each transponder read, or trip based, whereby a single user charge consisting of one or more transponder reads is recorded for each directional trip on the express toll lanes. For this analysis, trip based, TOD pricing is assumed for all Express Toll Lanes because it can be posted along the facility, documented in various forms of distributed user information and advertised through the media. This combination is expected to minimize potential confusion regarding the current price to use the toll facility, and thereby reduce the customer service center call volume. For open road tolling projects transactional based, fixed pricing is assumed.

Express Lane Access Points - Effectively, there are two types of access point designs, each with multiple implementation variation, for express lane projects. The two design alternatives are direct connections and slip ramps. Direct connections are accomplished at a common grade, involve a flyover of the general purpose lanes from a connecting ramp with touchdown at the express lane design grade or a tunnel under the general purpose lanes from a connecting ramp day-lighting at the express lane design grade. Unless the direct connection is at an express lane terminus or the direct connection adds or drops a continuous lane, the express lane facility must widen to provide a transition for users to merge or diverge from the express toll lanes to enter or exit, respectively. If the express lane facility is reversible, entry to the direct connection ramp or accessway must be controlled by multiple interspaced gates that are interlocked with gates controlling entry in the opposite direction.

Slip ramp access point design involves at grade entry to or exit from the general purpose lanes to the express toll lanes at either separate locations or at the same location along the express lane facility. Single point

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express lane entry requires an auxiliary lane diverging from the general purpose lanes and then merging within the express toll lanes along another auxiliary lane in accordance with design standards. Single point express lane exit requires an auxiliary lane diverging from the express toll lanes and then adjoining another auxiliary lane to merge with the general purpose. A variation of the slip ramp design is construction of a transition zone of sufficient length to handle weaving, merging and diverging for both express lane entry and exit from and to the general purpose lanes.

Field Support Structures - Physical implementation of ETC typically involves installation of gantry or bridge and cantilever structures above the mainline express toll lanes and access points, respectively, to mount antennae to transmit signals between either a small toll and communication building or roadside cabinet housing a ETC reader/controller and the vehicle mounted transponder. Gantry or bridge structures provide greater wind stability, which is particularly desirable for image quality from overhead violation enforcement cameras. Lower cost cantilever structures are also used for mounting signs.

Electronic Toll Collection Equipment - Express toll lanes are assumed as all-electronic facilities, with no provisions for manual or automatic coin machine collection. To assure interoperability, the transponder and other ETC equipment will be equivalent to the transponder and equipment used on E-470 and Northwest Parkway. Although one lane controller per lane provides high availability, express toll lanes will be deployed with one lane controller per direction to more efficiently handle cross lane reads, vehicles straddling two lanes and violation trigger messages.

Violation Enforcement Subsystem Equipment - Express toll lanes require the implementation of a violation enforcement subsystem (VES) to capture the license plates of vehicles that fail to record a valid transaction when traveling through a tolling point. This subsystem captures multiple license plate images of violating vehicles traveling in the express lane or adjacent shoulders, if sufficiently wide.

Optical character recognition (OCR) would be performed on the best of the multiple images to automatically extract the license plate characters and assign a level of confidence index to the extracted characters. OCR can be performed at the roadside level or centrally by providing equivalent processing capacity.

Express Lane Signing - Dynamic message signs (DMS) would typically be installed in advanced of the Express Toll Lane facility to notify prospective users of the approaching facility, locations serviced by the

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facility and selected pricing information. Notification of the current Express Toll Lane trip charges for selected destinations from decision points of diverting to the Express Toll Lanes can be made by overhead fixed, static signs with embedded changeable sign panels. Barrier mounted blankout signs can also be used to display trip charges in affect when entering the facility from various entry locations. Fixed static signs will also be used to inform users of approaching exits, posted speed limits, violator fines, TOD toll schedules, and other pertinent express lane information.

## Back Office Processing

- Host Computer Subsystem - The host computer system processes, stores and reports transactions and maintenance events received from the lane controllers. In turn, the host computer sends ETC and security account information, time synchronization and configuration data to the lane controllers. A primary function of the host computer system is to support the accounting and reconciliation process needed to accurately report revenues and expenses. The host computer system interfaces with a customer service and account management subsystem to send valid ETC transactions and receive transponder status lists and updates to the list.
- Customer Service and Account Management Subsystem - The customer service and account management subsystem supports the back-office operations for ETC including functions such as opening and closing an account, account management, transponder inventory and tracking, generating reports, and interfaces to a credit card clearinghouse, the violation processing subsystem, and the host computer subsystem. These functions would also be available from the a Web site and an interactive voice response (IVR) system. A call center will support customer calls regarding account establishment and management, ancillary issues related to the operation of the managed lanes calls from violators requesting information on violation citations.
- Violation Processing Subsystem - The violation processing subsystem processes violations using license plate images and violation transactions transmitted from the violation enforcement subsystem. This subsystem performs functions such as review and confirmation of video images, issuing tracking, and aging citations, processing payments, generating hearing evidence packages and interfacing with the Department of Motor Vehicles, the VES subsystem and customer service subsystem.

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## PROJECT EVALUATIONS

Presented below is a detailed narrative describing the project alternatives evaluated in the Denver area. For each project, traffic and revenue estimates are provided, along with a discussion identifying assumed project access and egress locations, tolling concepts, and optimum toll rates by time period and direction. These assumptions were used for this preliminary analysis, only. These project configuration assumptions will likely change as corridor studies progress. A total of 14 project alternatives along 7 corridors were evaluated in the Denver area. In four of the corridors multiple alternatives were evaluated.

## I-25 NORTH EXPRESS TOLL LANES - SCENARIOS 1 AND 2

Two scenarios were considered for this corridor. Scenario 1 is an approximately 26 mile express toll lane from S.H. 66 to U.S. 36. Scenario 2 is a 12 mile project which extends from S.H. 7 to U.S. 36. The detailed assumptions for each project, along with the analytical findings, are presented below.

The I-25 North Scenario 1 project spans approximately 26 miles between S.H. 66 and U.S. 36. For this analysis, the project was subdivided into two sections with different improvement types. From S.H. 66 to $120^{\text {th }}$ Avenue, I-25 was assumed to have three general purpose lanes and two express toll lanes in each direction. From $120^{\text {th }}$ to U.S. 36, the assumption was that I-25 would have three general purpose lanes in each direction and two reversible express toll lanes. A separate on-going study is looking at the feasibility of converting the existing two-lane reversible highoccupancy vehicle (HOV) facility from U.S. 36 to downtown Denver to a two-lane reversible high-occupancy toll (HOT) facility. This HOV to HOT conversion has been assumed in this analysis.

The I-25 North Scenario 2 project limits extend from S.H. 7 to U.S. 36, a distance of approximately 12 miles. From S.H. 7 to U.S. 36, I-25 was assumed to have three general purpose lanes in each direction and two reversible express toll lanes. As mentioned above, a separate on-going study is evaluating the feasibility of converting the existing two-lane reversible high-occupancy vehicle (HOV) facility from U.S. 36 to downtown Denver to a two-lane reversible high-occupancy toll (HOT) facility. This conversion has also been assumed in this scenario.

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## Current Traffic Conditions

For the analysis of these project scenarios, the latest regional traffic model was obtained from the DRCOG. The latest available hourly traffic counts at two mainline locations on I- 25 were used to aid in the development of existing and future year hourly traffic profiles. The two 2004 traffic count locations were situated between S.H. 52 and S.H. 119, and between $120^{\text {th }}$ and E-470/Northwest Parkway, where the weekday volumes were 75,000 and 85,300 , respectively.

Hourly traffic volumes at the two count locations show a generally balanced flow in either travel direction during the morning and evening peak periods, although at the northernmost location volumes are slightly higher in the southbound a.m. and northbound p.m. periods. At the southernmost count location, the southbound volumes are higher in the a.m. peak period. In the p.m. peak period northbound and southbound volumes are similar. These variations are based on a one day count in February. More extensive counts should be performed for a finance-grade study.

## Project Access Points And Tolling Concept

The assumed points of express toll lane access and tolling for Scenarios 1 and 2 are shown in Figure 3-2. Entry and exit access to the express toll lanes were assumed to be provided via nine locations in Scenario 1 and six locations in the shorter Scenario 2. In Scenario 1 partial access to/from the south was assumed at five locations including:

- North of S.H. 119;
- North of S.H. 52;
- North of S.H. 8;
- North of $144^{\text {th }}$ Avenue; and
- Ramps to/from U.S. 36.

Full directional access was assumed to be provided at three locations, one north of $120^{\text {th }}$ Avenue, another south of $104^{\text {th }}$ Avenue, and another south of E. $84^{\text {th }}$ Avenue. The final access location assumption provides slip ramps to/from the north south of S.H. 8. The transition from two reversible to four express lanes was assumed to be at the full access locations just north of $120^{\text {th }}$ Avenue.

In Scenario 2, except for slip ramps south of S.H. 7, providing the northernmost access to the express toll lanes, all other access points south of E-470 are the same as in Scenario 1. However, in this case, the entire project north to S.H. 7 would be two lane reversible.

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I-25 NORTH PROPOSED ETL ACCESS POINTS AND TOLLING ZONES

Figure 3-2 also identifies the general location of tolling zones for each scenario. There are seven tolling zones identified in Scenario 1 and four in Scenario 2. This number of tolling zones allows for tolls to be levied on a per-mile basis, which would be more equitable for motorists on these longer distance projects.

## Preliminary Traffic and Revenue Estimates

Presented below are the estimated optimum toll rates by time period which were the outcome of toll sensitivity analyses performed for the assumed opening year (2010) and future year (2025). Also presented below are estimates of average weekday traffic for years 2010 and 2025. Volumes shown are by time period and total weekday on the express toll lanes and total weekday on the general purpose lanes. Finally, estimates of the annual number of trips and gross toll revenue for each scenario are provided.

Toll Rates - A toll sensitivity analysis was conducted for each analysis period in years 2010 and 2025 for the two scenarios. A per-mile rate structure was assumed for both scenarios. Per-mile toll rates tested ranged from $\$ 0.05$ to $\$ 0.50$. Shown in Tables 3-2 and 3-3 are the optimum passenger car-based per-mile toll rates by time period and travel direction for Scenarios 1 and 2, respectively. Also presented is the toll for a 26 mile, full-length trip by time period and direction. The full-length tolls in Scenario 1 range from $\$ 0.70$ to $\$ 8.00$ in year 2010 and from $\$ 0.70$ to $\$ 10.00$ in 2025, depending on the time period. For the approximately 12 mile, full-length trip along the reversible toll lanes in Scenario 2, tolls could range from $\$ 1.20$ to $\$ 4.75$ in 2010, depending on the time period, and from $\$ 2.40$ to $\$ 6.60$ by 2025.

It should be noted that in Scenario 1, two per-mile toll rates are shown for each of the seven time periods. The lower rates apply to the three northernmost tolling zones and are 75 percent of the rates at the southern four tolling zones. This adjustment was applied to account for lower traffic volumes and congestion levels in the northern project segments.

Estimated Traffic - Average toll weekday traffic along I-25 on the general purpose and express toll lanes was summarized for each scenario at the optimum toll rates. Additionally, weekday volumes on the express toll lanes by time period are also provided. These volumes at 2010 and 2025 levels are displayed in Figures 3-3 and 3-4 for Scenario 1 and Figure 3-5 for Scenario 2.
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(1) Toll rates at the three northernmost tolling zones are 75 percent

$$
\begin{gathered}
\text { Table 3-2 } \\
\text { Toll Rates By Time Period } \\
\text { I-25 Express Toll Lanes } \\
\text { Scenario } 1: \text { U.S. } 36 \text { to S.H. } 66 \\
\text { (2004 dollars) }
\end{gathered}
$$

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Table 3-3
Toll Rates By Time Period
I-25 Express Toll Lanes
Scenario $2:$ U.S. 36 to S.H. 7
(2004 dollars)



| $\stackrel{0}{0}$ |  |
| :---: | :---: |


| $\quad$ Time Period |
| :--- |
|  |
| AM Shoulder |
| AM Peak |
| AM Shoulder |
| Midday |
| PM Shoulder |
| PM Peak |
| PM Shoulder |



| 2010 |  |
| :---: | :---: |
| AMPK | 3.8 |
| AMSH | 4.6 |
| MD | 0.0 |
| PMSH | 0.0 |
| PMPK | 0.0 |
| Daily | 8.4 |



From I-25 HOT Lanes
$\qquad$

|c|c| | 2025 |  |
| :---: | :---: |
| AMPK | 1.1 |
| AMSH | 1.1 |
| MD | 6.5 |
| PMSH | 2.1 |
| PMPK | 2.9 |
| Daily | 13.7 |

| 2025 |  |
| :---: | :---: |
| AMPK | 1.3 |
| AMSH | 1.4 |
| MD | 7.5 |
| PMSH | 2.5 |
| PMPK | 3.4 |
| Daily | 16.1 |



| 2025 |  |
| :---: | :---: |
| AMPK | 2.3 |
| AMSH | 2.8 |
| MD | 4.9 |
| PMSH | 2.3 |
| PMPK | 3.4 |
| Daily | 15.7 |



| 2025 |  |
| :---: | :---: |
| AMPK | 1.6 |
| AMSH | 1.7 |
| MD | 8.2 |
| PMSH | 2.8 |
| PMPK | 3.9 |
| Daily | 18.2 |



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In Scenario 1, the peak load point on the express toll lanes in year 2010 is 19,400 vehicles north of East $84^{\text {th }}$ Avenue. This volume represents about 9 percent of the total I- 25 demand at this location. The lowest load point on the express toll lanes is south of S.H. 7 where 8,300 vehicles use the facility. This represents about 6 percent of the total demand of 142,000 at this location. By 2025, the highest weekday express toll lane load point is located north of S.H. 8 with 36,600 vehicles or 22 percent of the total demand.

In Scenario 2, the peak load point on the express toll lanes in year 2010 is 19,000 vehicles north of East $84^{\text {th }}$ Avenue. As in Scenario 1, this volume represents about 9 percent of the total vehicular demand at this location. The lowest point is south of S.H. 7 with 3,900 vehicles in the express toll lanes. However, this point is at the end of the reversible express toll lane project. By 2025, the highest weekday express toll lane load point is also located north of East $84^{\text {th }}$ Avenue with 22,700 vehicles, or 9 percent of total vehicular demand.

Estimated Annual Trips And Gross Toll Revenue - Annual trips and gross toll revenue for Scenarios 1 and 2 are provided in Tables 3-4 and 35, respectively. Scenario 1 produces an estimated $\$ 15.2$ million in gross toll revenue in year 2010 and grows to an estimated $\$ 59.9$ million by year 2025. The annual number of trips is estimated at 10.4 million in year 2010, growing to 17.5 million by year 2025 .

Gross toll revenue produced by Scenario 2 is estimated at $\$ 10.0$ million in year 2010 and $\$ 23.2$ million in year 2025. The annul number of trips in the express toll lanes rises from 5.7 million in year 2010 to 7.0 million in year 2025.

In this and other express toll lane project scenarios, average weekday transactions and revenue for years 2010 and 2025 were annualized. Annual revenue for 2010 and 2025 were calculated using 200 interior weekdays (Monday through Thursday), 52 Fridays, and 113 weekend days. Fridays were assumed to have 20 percent more revenue than the average weekday, and weekend days were assumed to represent 25 percent of the revenue on an average weekday. These Friday and weekend assumptions for I-25 were used since Friday traffic is generally greater than the average weekday, producing greater levels of congestion. Therefore, the revenue generating potential of the project on Friday's would be greater than the average weekday. Conversely, weekend traffic is generally lower and limited use of express toll lanes would be anticipated. Annual transactions were similarly calculated. Intermediate year transactions and revenue were calculated through interpolation
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Table 3-4
Annual Traffic And Revenue Estimates
I-25 Express Toll Lanes
Scenario 1 : U.S. 36 to S.H. 66

| Year | Annual Trips (1) | Annual Revenue (1,2) |
| :---: | :---: | :---: |
|  | (000) | (000) |
| 2010 | 10,376 | \$15,192 |
| 2011 | 10,745 | 16,647 |
| 2012 | 11,127 | 18,242 |
| 2013 | 11,523 | 19,989 |
| 2014 | 11,933 | 21,904 |
| 2015 | 12,357 | 24,002 |
| 2016 | 12,796 | 26,301 |
| 2017 | 13,252 | 28,820 |
| 2018 | 13,723 | 31,581 |
| 2019 | 14,211 | 34,606 |
| 2020 | 14,716 | 37,921 |
| 2021 | 15,240 | 41,553 |
| 2022 | 15,782 | 45,534 |
| 2023 | 16,343 | 49,895 |
| 2024 | 16,924 | 54,675 |
| 2025 | 17,526 | 59,912 |
| 2026 | 17,964 | 62,308 |
| 2027 | 18,413 | 64,801 |
| 2028 | 18,874 | 67,393 |
| 2029 | 19,345 | 70,089 |
| 2030 | 19,829 | 72,892 |
| 2031 | 20,226 | 75,079 |
| 2032 | 20,630 | 77,331 |
| 2033 | 21,043 | 79,651 |
| 2034 | 21,464 | 82,041 |
| 2035 | 21,893 | 84,502 |
| 2036 | 22,112 | 86,192 |
| 2037 | 22,333 | 87,916 |
| 2038 | 22,556 | 89,674 |
| 2039 | 22,782 | 91,468 |
| 2040 | 23,010 | 93,297 |



Table 3-5
Annual Traffic And Revenue Estimates
I-25 Express Toll Lanes
Scenario 2 : U.S. 36 to S.H. 7

| Year | Annual Trips (1) |  |
| :--- | :---: | :---: |
|  | $(000)$ | Annual Revenue (1, 2) |
|  |  | $(000)$ |
| 2010 | 5,668 |  |
| 2011 | 5,750 | $\$ 10,014$ |
| 2012 | 5,834 | 10,591 |
| 2013 | 5,918 | 11,201 |
| 2014 | 6,004 | 11,847 |
| 2015 | 6,091 | 12,530 |
| 2016 | 6,179 | 13,252 |
| 2017 | 6,269 | 14,016 |
| 2018 | 6,360 | 14,823 |
| 2019 | 6,452 | 15,678 |
| 2020 | 6,546 | 16,581 |
| 2021 | 6,640 | 17,537 |
| 2022 | 6,737 | 18,547 |
| 2023 | 6,834 | 19,616 |
| 2024 | 6,933 | 20,747 |
| 2025 | 7,034 | 21,942 |
| 2026 | 7,132 | 23,207 |
| 2027 | 7,232 | 24,135 |
| 2028 | 7,334 | 25,101 |
| 2029 | 7,436 | 26,105 |
| 2030 | 7,540 | 27,149 |
| 2031 | 7,646 | 28,235 |
| 2032 | 7,753 | 29,082 |
| 2033 | 7,862 | 29,954 |
| 2034 | 7,972 | 30,853 |
| 2035 | 8,083 | 31,779 |
| 2036 | 8,164 | 32,732 |
| 2037 | 8,246 | 33,387 |
| 2038 | 8,411 | 34,054 |
| 2039 |  | 35,435 |
| 2040 |  | 36,139 |
|  |  |  |

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between 2010 and 2025. From 2025 to 2030 a 4.0 percent increase in annual revenue was assumed, reducing to 3.0 percent between 2030 and 2035, and 2.0 percent from 2035 to 2040.

Similar assumptions and procedures were used to annualize transactions and revenue for other projects evaluated, although there may have been slight variations used for the assumed weekday to Friday, and weekday to weekend day percentages.

## I-70 EAST EXPRESS TOLL LANES - SCENARIOS 1, 2 AND 3

Three I-70 east express toll lane scenarios were evaluated. The scenarios are:

- Scenario 1 - A 12-mile express toll lane project between I-25 and E470, with the express toll lanes on elevated structure between I-25 and I-270;
- Scenario 2 - A 9-mile express toll lane project between I-25 and Chambers Road, with the express toll lanes on elevated structure between I-25 and I-270; and
- Scenario 3 - A 6-mile express toll lane project between Colorado Boulevard and Chambers Road, without the need for any portion of the express toll lanes to be on elevated structure. The detailed assumptions for each project, along with the analytical findings are presented below.

The I-70 East Scenario 1 project is located between I-25 and E-470. The project is approximately 12 miles and is subdivided into two sections with different improvement types. From I-25 to just east of I-270, the section would have three general purpose lanes in each direction, the majority of which is on elevated structure, and two express toll lanes each direction on elevated structure, located adjacent to the existing I-70 alignment on the north side. From just east of I-270 to just west of E-470, I-70 would vary between two (east of Chambers Road to E-470) and four (east of I-270 to east of Chambers Road) general purpose lanes, plus two express toll lanes in each direction located at-grade.

The I-70 East Scenario 2 project is located between I-25 and Chambers Road. The project is approximately nine miles and is subdivided into two sections with different improvement types. From I-25 to just east of I-270, the section would have three general purpose lanes in each direction, the

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majority of which is on elevated structure, and two express toll lanes each direction on elevated structure, located adjacent to the existing I-70 alignment on the north side. From just east of I-270 to Chambers, I-70 would have four general purpose lanes and two express toll lanes in each direction located at-grade. Within this section the express toll lanes are assumed to be located in the median of existing I-70 and separated from the general purpose lanes by a concrete barrier.

The I-70 East Scenario 3 project is located between Colorado Boulevard and Chambers Road. The project is approximately six miles and would not have an elevated structure as was assumed in Scenarios 1 and 2. From just east of Colorado Boulevard to I-270, I-70 is assumed to have three general purpose lanes and two express toll lanes in each direction located at-grade. Within this section the express toll lanes are assumed to be located in the median of existing I-70 and separated from the general purpose lanes by a concrete barrier. It was assumed that the existing general purpose lanes would need to be reconstructed between Colorado and Chambers because the current median width is not sufficient to add express toll lanes in the median without impacting the general purpose lanes.

## Current Traffic Conditions

For the analysis of these scenarios the latest regional traffic model was obtained for the DRCOG. A collection of hourly traffic counts on all I-70 ramps within the study corridor and at two mainline locations along I-70 were used to create an hourly traffic demand profile for 2002. As expected, the highest volume occurs east of the I-270 ramps where the average weekday traffic is 175,000 vehicles across the eight lane crosssection. West of the I-270 ramps, traffic is heaviest between Brighton Boulevard and York Street at 140,000 vehicles across the six lane crosssection. Traffic levels drop off significantly east of Airport Road to 37,200 vehicles.

Hourly variation patterns along the mainline sections of I-70 showed that the west end of I-70 is the only significant location showing directional peaking patterns. There, traffic is heaviest in the morning eastbound and westbound during the afternoon peak. East of the I-270 ramps both directions are heavily used during the peaks with the westbound peaks starting earlier than the eastbound peaks. There is a significant movement of traffic between I-270 and I-225, along I-70.

## Project Access Points And Tolling Concept

The assumed points of express toll lane access and tolling for Scenarios 1, 2 and 3 are shown in Figure 3-6. Entry to and exit from the express toll lanes are assumed via seven locations in Scenario 1, six locations in

## SCENARIO 1: I-25 TO E-470



SCENARIO 2: I-25 TO CHAMBERS ROAD


SCENARIO 3: COLORADO BOULEVARD TO CHAMBERS ROAD


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Scenario 2, and five locations in the shorter Scenario 3. In Scenario 1 access is assumed to be provided at the following locations:

- To/from the east, west of Brighton Boulevard;
- To/from the east at Colorado Boulevard;
- To/from the east at Quebec Street;
- To/from the east with direct connection ramps with I-270;
- To/from the west with direct connection ramps with I-225;
- To/from the west, west of Chambers Road; and
- To/from the west, west of E-470.

In Scenario 2, the only assumed access difference is the removal of the access to/from the west, west of E-470 since the project ends at a location west of Chambers Road.

Access in Scenario 3, is assumed to be identical to Scenario 2, with the exception of the removal of the far west access, west of Brighton Boulevard. The project is assumed to begin east of Colorado Boulevard.

Figure 3-6 also identifies the general location of tolling zones for each scenario. There are three tolling zones identified in Scenario 1 and two in Scenarios 2 and 3. Under Scenario 1, the far west tolling zone would cover the distance traveled to the I-270 access, the middle tolling zone would cover the distance traveled from the I-270 access to Chambers Road and the last tolling zone would cover the distance from Chambers Road to E-470. Under Scenarios 2 and 3, the last tolling zone is not needed.

## Preliminary Traffic and Revenue Estimates

Presented below are the estimated optimum toll rates by time period which were the outcome of toll sensitivity analyses performed for the assumed opening year (2010) and future year (2025). Also presented below are estimates of average weekday traffic for years 2010 and 2025 . Volumes shown are by time period and total weekday on the express toll lanes and total weekday on the general purpose lanes. Finally, estimates of the annual number of trips and gross toll revenue for each scenario are provided.

Toll Rates - A toll sensitivity analysis was conducted for each analysis period in years 2010 and 2025 for the three scenarios. A per-mile rate structure was assumed for all three scenarios. Per-mile toll rates tested ranged from $\$ 0.05$ to $\$ 0.50$. Shown in Tables $3-6,3-7$ and $3-8$ are the optimum passenger car-based per-mile toll rates by time period and travel direction for Scenarios 1, 2 and 3, respectively. Also presented is the toll

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Table 3-7
Toll Rates By Time Period
I-70 Express Toll Lanes
Scenario 2: I-25 to Chambers Road
(2004 dollars)

| 2010 |  |  |  | 2025 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Eastbound |  | Westbound |  | Eastbound |  | Westbound |  |
| Per-Mile | Through Trip | Per-Mile | Through Trip | Per-Mile | Through Trip | Per-Mile | Through Trip |
| \$0.10 | \$0.80 | \$0.13 | \$0.95 | \$0.25 | \$1.95 | \$0.18 | \$1.35 |
| 0.425 | 3.25 | 0.350 | 2.70 | 0.550 | 4.25 | 0.475 | 3.70 |
| 0.300 | 2.30 | 0.125 | 0.95 | 0.425 | 3.25 | 0.250 | 1.95 |
| 0.100 | 0.80 | 0.075 | 0.60 | 0.125 | 0.95 | 0.125 | 0.95 |
| 0.300 | 2.30 | 0.250 | 1.95 | 0.375 | 2.90 | 0.450 | 3.45 |
| 0.425 | 3.25 | 0.350 | 2.70 | 0.550 | 4.25 | 0.550 | 4.25 |
| 0.150 | 1.15 | 0.075 | 0.60 | 0.225 | 1.75 | 0.300 | 2.30 |



| Time Period |
| :--- |
|  |
| AM Shoulder |
| AM Peak |
| AM Shoulder |
| Midday |
| PM Shoulder |
| PM Peak |
| PM Shoulder |

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Toll Rates By Time Period

| Time Period | Hours | 2010 |  |  |  | 2025 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Eastbound |  | Westbound |  | Eastbound |  | Westbound |  |
|  |  | $\underline{\text { Per-Mile }}$ | Through Trip | Per-Mile | Through Trip | Per-Mile | Through Trip | Per-Mile | Through Trip |
| AM Shoulder | 6-7 a.m. | \$0.10 | \$0.60 | \$0.13 | \$0.70 | \$0.25 | \$1.45 | \$0.18 | \$1.00 |
| AM Peak | 7-8 a.m. | 0.425 | 2.40 | 0.350 | 2.00 | 0.550 | 3.15 | 0.475 | 2.75 |
| AM Shoulder | 8-9 a.m. | 0.300 | 1.70 | 0.125 | 0.70 | 0.425 | 2.40 | 0.250 | 1.45 |
| Midday | 9 a.m. - 3 p.m. | 0.100 | 0.60 | 0.075 | 0.45 | 0.125 | 0.70 | 0.125 | 0.70 |
| PM Shoulder | 3-4 p.m. | 0.300 | 1.70 | 0.250 | 1.45 | 0.375 | 2.15 | 0.450 | 2.55 |
| PM Peak | 4-6 p.m. | 0.425 | 2.40 | 0.350 | 2.00 | 0.550 | 3.15 | 0.550 | 3.15 |
| PM Shoulder | 6-7 p.m. | 0.150 | 0.85 | 0.075 | 0.45 | 0.250 | 1.45 | 0.300 | 1.70 |

for a full-length trip by time period and direction. The full-length tolls in Scenario 1 range from $\$ 1.00$ to $\$ 6.20$ in year 2010 and from $\$ 1.60$ to $\$ 7.15$ in 2025, depending on the time period. In Scenario 2, a full-length trip along the express toll lanes would range from $\$ 0.60$ to $\$ 3.25$ in 2010, depending on the time period, and from $\$ 0.95$ to $\$ 4.25$ by 2025 . In Scenario 3, a full-length trip along the express toll lanes would range from $\$ 0.45$ to $\$ 2.40$ in 2010, and from $\$ 0.70$ to $\$ 3.15$ by 2025.

Estimated Traffic - Average toll weekday traffic along I-70 on the general purpose and express toll lanes was summarized for each scenario at the optimum toll rates. Additionally, weekday volumes on the express toll lanes by time period are also provided. These volumes at 2010 and 2025 levels are displayed in Figures 3-7 and 3-8 for Scenario 1, Figures 39 and 3-10 for Scenario 2, and Figures 3-11 and 3-12 for Scenario 3.

In reviewing new traffic estimates for the I-70 alternatives, and all of the Express Toll Lane type projects, it should be noted that traffic estimates are shown by analysis time period. The a.m. peak period, for analysis purposes, represents a one hour period, while the a.m. shoulder period represents two hours. Midday represents six hours, while p.m. shoulder and p.m. peak periods represent two hours each. The traffic volumes shown in the following figures, represent the total volumes for the entire period.

For example, in the traffic estimates shown in Figure 3-7, at the tolling zone located east of I-270, the eastbound a.m. peak hour traffic is estimated at 2,800 vehicles. The a.m. shoulder is estimated at 3,000 vehicles, over two hours, 1,750 vehicles per hour. The midday volume is estimated at 8,000 vehicles, but represents six hours; with an average of slightly more than 1,300 vehicles per hour.

It also should be kept in mind that highly variable toll rates are assumed to be used, rates which would optimize revenue while still managing freeflow traffic in peak periods. The toll rate in the midday is one fourth of the rate which would be charged in peak hour conditions; hence midday traffic usage would be lower on a per-hour basis and revenues would be much lower.

In Scenario 1, the peak load point on the express toll lanes in year 2010 is 43,300 vehicles east of I-270 access points. All traffic using the facility has to pass through this point and therefore would have the largest volume. This volume represents about 17 percent of the total I-70 demand at this location. At this same location during the a.m. peak hour, the estimated eastbound volume is 2,800 vehicles on the two express toll





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Note: All volumes shown represent thousands of vehicles.

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Note: All volumes shown represent thousands of vehicles.

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Note: All volumes shown represent thousands of vehicles.

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Note: All volumes shown represent thousands of vehicles.

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lanes. By 2025 at this same location, the weekday express toll lane volume is estimated to increase to 65,300 or 23 percent of the total demand. Significant movements between the express toll lanes and the direct connection ramps to I-270 and I-225 are shown. In Scenario 2, similar volumes are shown as compared to Scenario 1. The section of managed lanes east of Chambers Road under Scenario 1 was found not to provide significant added benefit. This section of toll road actually would tend to increase the cost of a through trip with no real added benefit since the time savings advantage of the toll lanes would likely have already occurred west of Chambers Road. Weekday express lane volumes at the peak load point are estimated at 45,500 vehicles in 2010 and growing to 67,000 by 2025.

Scenario 3 would have slightly lower volumes than compared with Scenario 2 because of the shorter project and removal of the far west access. The peak load point shows the toll lanes would still be heavily used, especially within the peak periods.

Estimated Annual Trips And Gross Toll Revenue - Annual trips and gross toll revenue for Scenarios 1, 2, and 3 are provided in Tables 3-9, 310, and 3-11, respectively. Scenario 1 produces an estimated \$17.5 million in gross toll revenue in year 2010 and grows to an estimated $\$ 44.5$ million by year 2025. The annual number of trips is estimated at 12.6 million in year 2010, growing to 18.7 million by year 2025.

Gross toll revenue produced by Scenario 2 is estimated at $\$ 17.3$ million in year 2010 and $\$ 40.4$ million in year 2025. The annul number of trips in the express toll lanes rises from 12.1 million in year 2010 to 18.2 million in year 2025 .

Scenario 3 is estimated to produce $\$ 15.9$ million in gross toll revenue in 2010, growing to $\$ 37.9$ million by 2025. The annual number of trips for 2010 is estimated at 12.3 million and is estimated to increase to 18.0 million by year 2025. It is interesting to note that the annual revenue growth from 2010 to 2015 is double that of the trip growth on the toll lanes. This is due to toll rate increases which can occur as demand continues to grow within the corridor.

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Table 3-9
Annual Traffic And Revenue Estimates
I-70 Express Toll Lanes
Scenario 1 : I-25 to E-470

| Year | Annual Trips (1) | Annual Revenue (1, 2) |
| :---: | :---: | :---: |
|  | (000) | (000) |
| 2010 | 12,571 | \$17,488 |
| 2011 | 12,908 | 18,612 |
| 2012 | 13,254 | 19,808 |
| 2013 | 13,609 | 21,081 |
| 2014 | 13,973 | 22,436 |
| 2015 | 14,347 | 23,878 |
| 2016 | 14,732 | 25,413 |
| 2017 | 15,126 | 27,047 |
| 2018 | 15,532 | 28,785 |
| 2019 | 15,948 | 30,635 |
| 2020 | 16,375 | 32,604 |
| 2021 | 16,814 | 34,700 |
| 2022 | 17,264 | 36,930 |
| 2023 | 17,727 | 39,303 |
| 2024 | 18,201 | 41,829 |
| 2025 | 18,689 | 44,518 |
| 2026 | 19,063 | 46,299 |
| 2027 | 19,444 | 48,151 |
| 2028 | 19,833 | 50,077 |
| 2029 | 20,230 | 52,080 |
| 2030 | 20,634 | 54,163 |
| 2031 | 20,944 | 55,788 |
| 2032 | 21,258 | 57,461 |
| 2033 | 21,577 | 59,185 |
| 2034 | 21,900 | 60,961 |
| 2035 | 22,229 | 62,790 |
| 2036 | 22,451 | 64,046 |
| 2037 | 22,676 | 65,326 |
| 2038 | 22,902 | 66,633 |
| 2039 | 23,131 | 67,966 |
| 2040 | 23,363 | 69,325 |

[^1]

Table 3-10
Annual Traffic And Revenue Estimates

## I-70 Express Toll Lanes

Scenario 2 : I-25 to Chambers

| Year | Annual Trips (1) | Annual Revenue (1, 2) |
| :---: | :---: | :---: |
|  | (000) | (000) |
| 2010 | 12,096 | \$17,273 |
| 2011 | 12,429 | 18,280 |
| 2012 | 12,772 | 19,345 |
| 2013 | 13,124 | 20,472 |
| 2014 | 13,486 | 21,666 |
| 2015 | 13,857 | 22,928 |
| 2016 | 14,239 | 24,265 |
| 2017 | 14,631 | 25,679 |
| 2018 | 15,035 | 27,175 |
| 2019 | 15,449 | 28,759 |
| 2020 | 15,875 | 30,435 |
| 2021 | 16,312 | 32,209 |
| 2022 | 16,762 | 34,086 |
| 2023 | 17,224 | 36,073 |
| 2024 | 17,698 | 38,175 |
| 2025 | 18,186 | 40,400 |
| 2026 | 18,550 | 42,016 |
| 2027 | 18,921 | 43,697 |
| 2028 | 19,299 | 45,445 |
| 2029 | 19,685 | 47,262 |
| 2030 | 20,079 | 49,153 |
| 2031 | 20,280 | 50,627 |
| 2032 | 20,482 | 52,146 |
| 2033 | 20,687 | 53,711 |
| 2034 | 20,894 | 55,322 |
| 2035 | 21,103 | 56,982 |
| 2036 | 21,209 | 58,121 |
| 2037 | 21,315 | 59,284 |
| 2038 | 21,421 | 60,469 |
| 2039 | 21,528 | 61,679 |
| 2040 | 21,636 | 62,912 |

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Table 3-11
Annual Traffic And Revenue Estimates
I-70 Express Toll Lanes
Scenario 3 : Colorado to Chambers

| Year | Annual Trips (1) | Annual Revenue (1, 2) |
| :---: | :---: | :---: |
|  | (000) | (000) |
| 2010 | 12,263 | \$15,905 |
| 2011 | 12,582 | 16,852 |
| 2012 | 12,909 | 17,855 |
| 2013 | 13,244 | 18,917 |
| 2014 | 13,588 | 20,043 |
| 2015 | 13,941 | 21,236 |
| 2016 | 14,303 | 22,500 |
| 2017 | 14,675 | 23,839 |
| 2018 | 15,056 | 25,258 |
| 2019 | 15,448 | 26,762 |
| 2020 | 15,849 | 28,355 |
| 2021 | 16,261 | 30,042 |
| 2022 | 16,683 | 31,830 |
| 2023 | 17,117 | 33,725 |
| 2024 | 17,562 | 35,732 |
| 2025 | 18,018 | 37,859 |
| 2026 | 18,288 | 39,373 |
| 2027 | 18,563 | 40,948 |
| 2028 | 18,841 | 42,586 |
| 2029 | 19,124 | 44,290 |
| 2030 | 19,411 | 46,061 |
| 2031 | 19,605 | 47,443 |
| 2032 | 19,801 | 48,866 |
| 2033 | 19,999 | 50,332 |
| 2034 | 20,199 | 51,842 |
| 2035 | 20,401 | 53,398 |
| 2036 | 20,503 | 54,466 |
| 2037 | 20,605 | 55,555 |
| 2038 | 20,708 | 56,666 |
| 2039 | 20,812 | 57,799 |
| 2040 | 20,916 | 58,955 |

[^3]
## U.S. 36 EXPRESS TOLL LANES - SCENARIO 1

The U.S. 36 project extends from Foothills Parkway near the city limits of Boulder to the eastern terminus at I-25. The project is approximately 18 miles long and is subdivided into three sections with different improvement types. From Foothills Parkway to McCaslin Boulevard, the section is assumed to have two general purpose lanes and one express toll lane each direction. From McCaslin Boulevard to Pecos, it is assumed that U.S. 36 would have two general purpose lanes and two express toll lanes in each direction and from Pecos to I-25, the project assumes the conversion of the existing one-lane reversible high-occupancy vehicle (HOV) facility to a two-lane reversible high-occupancy toll (HOT) facility. The section is assumed to have two general purpose lanes in each direction.

## Current Traffic Conditions

For the analysis of these project scenarios the latest regional traffic model was obtained from the DRCOG. The latest available hourly traffic counts at two mainline locations on U.S. 36, one south of McCaslin Boulevard and one south of Wadsworth Parkway, along with hourly counts at all existing ramps, were used to aid in the development of existing and future year hourly traffic profiles. Weekday volumes at the two mainline count locations were 82,700 vehicles south of McCaslin Boulevard and 87,700 vehicles south of Wadsworth Parkway. Average weekday volumes along U.S. 36 are highest at the south end between I- 25 and Sheridan Boulevard ranging from 110,000 to 140,000 vehicles, between Sheridan and McCaslin Boulevards traffic drops to between 75,000 and 90,000 vehicles, and north of McCaslin Boulevard volumes are in the 50,000 vehicle range.

Hourly traffic volume counts from the two mainline count locations show a typical condition of peak traffic flow southbound during the morning peak period, with generally higher volumes northbound during the evening peak hours. Southbound a.m. peak hour volumes are generally in the range of 3,500 vehicles at both count locations, with similar but slightly higher volumes during the northbound p.m. peak hours.

## Project Access Points And Tolling Concept

The assumed points of express toll lane access and tolling for Scenario 1 are shown in Figure 3-13. Entry to and exit from the express toll lanes are assumed to be made via seven locations. In addition to the slip ramp access/egress at both the north and south ends of the express toll lanes, full directional access is currently assumed at five locations including:
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- South of McCaslin Boulevard;
- North of Wadsworth Parkway;
- North of $92^{\text {nd }}$ Avenue;
- North of Federal Boulevard; and
- North of Broadway.

Figure 3-13 also identifies the general location of tolling zones. There are five tolling zones identified. This number of tolling zones allows for tolls to be levied on a per-mile basis, which would be more equitable for motorists on this approximately 17 mile project.

## Preliminary Traffic and Revenue Estimates

Presented below are the estimated optimum toll rates by time period which were the outcome of toll sensitivity analyses performed for the assumed opening year (2010) and future year (2025). Also presented below are estimates of average weekday traffic for years 2010 and 2025. Volumes shown are by time period and total weekday on the express toll lanes and total weekday on the general purpose lanes. Finally, estimates of the annual number of trips and gross toll revenue for each scenario are provided.

Toll Rates - A toll sensitivity analysis was conducted for each analysis period in years 2010 and 2025. A per-mile rate structure was assumed for the facility. Per-mile toll rates tested ranged from $\$ 0.050$ to $\$ 0.500$. Shown in Table 3-12 is the optimum passenger car-based per-mile toll rate by time period and travel direction. Also presented is the toll for an approximately 17 mile, full-length trip by time period and direction. The full-length tolls range from $\$ 0.90$ to $\$ 5.35$ in year 2010 and from $\$ 2.25$ to $\$ 8.05$ in 2025, depending on the time period.

Estimated Traffic - Average total weekday traffic along U.S. 36 on the general purpose and express toll lanes was summarized for the optimum toll rates. Additionally, weekday volumes on the express toll lanes by time period are also provided. These volumes at 2010 and 2025 levels are displayed in Figures 3-14 and 3-15, respectively.

The peak load point on the express toll lanes in year 2010 is 23,800 vehicles north of Federal Boulevard. This volume represents about 16 percent of the total U.S. 36 traffic demand at this location. The lowest load point on the express toll lanes is south of McCaslin Boulevard where 9,200 vehicles use the facility. This represents about 9 percent of the total demand of 109,000 vehicles at this location. By 2025, the highest weekday express toll lane load point is still located north of Federal Boulevard with 40,100 vehicles or 22 percent of the total demand of

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| Time Period | Hours | Table 3-12 <br> Toll Rates By Time Period U.S. 36 Express Toll Lanes I-25 to Foothills Parkway (2004 dollars) |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 2010 |  |  |  | 2025 |  |  |  |
|  |  | Eastbound |  | Westbound |  | Eastbound |  | Westbound |  |
|  |  | Per-Mile | $\begin{gathered} \text { Through } \\ \text { Trip } \\ \hline \end{gathered}$ | Per-Mile | $\begin{gathered} \text { Through } \\ \text { Trip } \\ \hline \end{gathered}$ | Per-Mile | $\begin{gathered} \text { Through } \\ \text { Trip } \\ \hline \end{gathered}$ | Per-Mile | $\begin{gathered} \text { Through } \\ \text { Trip } \\ \hline \end{gathered}$ |
| AM Shoulder | 6-7 a.m. | \$0.130 | \$2.15 | \$0.05 | \$0.90 | \$0.23 | \$5.00 | \$0.10 | \$2.25 |
| AM Peak | 7-8 a.m. | 0.300 | 5.00 | 0.225 | 4.00 | 0.450 | 8.00 | 0.450 | 8.00 |
| AM Shoulder | 8-9 a.m. | 0.175 | 3.15 | 0.150 | 2.70 | 0.270 | 6.00 | 0.225 | 5.00 |
| Midday | 9 a.m. - 3 p.m. | 0.075 | 1.35 | 0.050 | 0.90 | 0.125 | 2.70 | 0.100 | 2.25 |
| PM Shoulder | 3-4 p.m. | 0.150 | 2.70 | 0.150 | 2.70 | 0.250 | 5.50 | 0.275 | 6.00 |
| PM Peak | 4-6 p.m. | 0.250 | 4.50 | 0.275 | 5.00 | 0.450 | 8.00 | 0.450 | 8.00 |
| PM Shoulder | 6-7 p.m. | 0.100 | 1.80 | 0.100 | 1.80 | 0.225 | 5.00 | 0.225 | 5.00 |




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184,300 vehicles. The lowest load point remains south of McCaslin Boulevard where 13,800 vehicles use the facility. This represents about 11 percent of the total demand of 129,600 vehicles.

Estimated Annual Trips And Gross Toll Revenue - Annual trips and gross toll revenue for Scenario 1 are provided in Table 3-13. The U.S. 36 express toll lanes are estimated to generate $\$ 13.9$ million in gross toll revenue in year 2010, increasing to an estimated $\$ 40.1$ million by the year 2025. The annual number of trips is estimated at 11.4 million in year 2010, growing to 17.5 million by year 2025 .

## I-225 EXPRESS TOLL LANES - SCENARIO 1

The I-225 project spans approximately eight miles from I-70 to Parker Road (S.H. 83) and was assumed to consist of two express toll lanes and two general purpose lanes in each direction. The express toll lanes were assumed to be located in the median of the existing roadway and separated from the general purpose lanes by a concrete barrier. The section of the corridor from Parker Road to 6th Avenue has received environmental clearance for constructing six general purpose lanes and is included in the current TIP program; however, the project has not been implemented due to a lack of funding. For the purposes of this study, it was assumed that the improvements identified in the 2000 Environmental Assessment would be implemented in conjunction with the express toll lanes with the exception that only four general purpose lanes would be reconstructed instead of six from Parker Road to $6^{\text {th }}$ Avenue, as originally planned. North of $6^{\text {th }}$ Avenue, a total of six general purpose lanes and four express toll lanes were assumed.

## Current Traffic Conditions

For the study of this project, 48-hour interior weekday traffic count information collected over the course of several days in June and July 2002 were used to develop a profile of existing traffic conditions in the corridor. This was the latest data available that covered the entire corridor. Although mainline traffic counts were not collected, a full set of ramp counts at I-70 and I-25 interchanges and all interchanges in between allowed for development of mainline volumes through the entire corridor through addition and subtraction of ramp volumes.

Based on the data collected, total mainline traffic volumes were fairly uniform in the northern half of the corridor, varying from approximately 120,000 to 130,000 vehicles per weekday between I-70 and Alameda Avenue. South of this section, traffic tended to be slightly lower in the
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Table 3-13
Annual Traffic And Revenue Estimates
U.S. 36 Express Toll Lanes

I-25 to Foothills Parkway

| Year | Annual Trips (1) | Annual Revenue (1, 2) |
| :---: | :---: | :---: |
|  | (000) | (000) |
| 2010 | 11,423 | \$13,871 |
| 2011 | 11,751 | 14,888 |
| 2012 | 12,089 | 15,980 |
| 2013 | 12,436 | 17,151 |
| 2014 | 12,793 | 18,409 |
| 2015 | 13,160 | 19,758 |
| 2016 | 13,538 | 21,207 |
| 2017 | 13,927 | 22,762 |
| 2018 | 14,327 | 24,431 |
| 2019 | 14,739 | 26,222 |
| 2020 | 15,162 | 28,144 |
| 2021 | 15,597 | 30,208 |
| 2022 | 16,045 | 32,423 |
| 2023 | 16,506 | 34,800 |
| 2024 | 16,980 | 37,351 |
| 2025 | 17,468 | 40,090 |
| 2026 | 17,905 | 41,694 |
| 2027 | 18,352 | 43,361 |
| 2028 | 18,811 | 45,096 |
| 2029 | 19,281 | 46,900 |
| 2030 | 19,763 | 48,776 |
| 2031 | 20,159 | 50,239 |
| 2032 | 20,562 | 51,746 |
| 2033 | 20,973 | 53,298 |
| 2034 | 21,393 | 54,897 |
| 2035 | 21,820 | 56,544 |
| 2036 | 22,039 | 57,675 |
| 2037 | 22,259 | 58,829 |
| 2038 | 22,482 | 60,005 |
| 2039 | 22,706 | 61,205 |
| 2040 | 22,933 | 62,429 |

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100,000 to 115,000 vehicles range. Hourly traffic variations at two locations on I-225 were assessed. The first was between Colfax Avenue and $6^{\text {th }}$ Avenue and the second between Mississippi Avenue and Iliff Avenue. The first location was the highest volume link on the freeway, based on the aforementioned traffic counts.

For the two locations, northbound and southbound traffic levels were very similar to each other for most hours of the day. There was a little more directional peaking near the northern end of I-225, but in the central part of the corridor, the traffic volumes were almost equal for almost all hours of the day. For example, the directional split at the mainline section near Colfax Avenue was 55 percent northbound and 45 percent southbound during the morning peak hour, and 51 percent southbound and 49 percent northbound during the afternoon peak hour.

The hourly variations also tended to be fairly flat. The hour with the highest volume during the morning peak period accounted for 7.0 percent of travel in the northbound direction at the Colfax Avenue location. With the exception of two hours, each hour of the 13-hour period from 6:00 a.m. to 7:00 p.m., represented 5.0 to 6.5 percent of the total daily volume at this location.

However, these characteristics could change based on recent highway construction on I-225 and I-25, along with existing and planned land use developments in the corridor.

## Project Access Points And Tolling Concept

The assumed points of express toll lane access and tolling for Scenario 1 are shown in Figure 3-16. Entry to and exit from the express toll lanes were assumed to be made via five locations. Southbound entry/northbound exit ramps were assumed between I-70 and Colfax Avenue, and between Colfax Avenue and $6{ }^{\text {th }}$ Avenue. In addition, southbound exit/northbound entry ramps were assumed between Iliff and Mississippi Avenues, Parker Road and Iliff Avenue, and between Parker Road and Yosemite Street.

Figure 3-16 also identifies the general location of the proposed tolling zone. Only one tolling point was identified since this is a relatively short project with closely spaced interchanges. Toll rate inequities for short distance trips is sometimes a problem encountered with this type of toll collection system on longer projects, where long-distance trips would pay significantly less on a per-mile basis than short-distance trips. These inequities should not be as much a problem on this approximately eight mile project.


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Presented below are the estimated optimum toll rates by time period which were the outcome of toll sensitivity analyses performed for the assumed opening year (2010) and future year (2025). Also presented below are estimates of average weekday traffic for years 2010 and 2025. Volumes shown are by time period and total weekday on the express toll lanes and total weekday on the general purpose lanes. Finally, estimates of the annual number of trips and gross toll revenue for each scenario are provided.

Toll Rates - A toll sensitivity analysis was conducted for each analysis period in years 2010 and 2025. A flat toll rate structure was assumed for the facility. Toll rates tested ranged from $\$ 0.50$ to $\$ 3.00$. Shown in Table $3-14$ is the optimum passenger car-based toll by time period and direction. The toll has also been shown on a per-mile basis by dividing the optimum toll by the approximately eight mile full project length. The optimum tolls range from $\$ 0.75$ to $\$ 2.00$ in year 2010 and from $\$ 1.50$ to $\$ 3.00$ in 2025, depending on the time period.

Estimated Traffic - Average total weekday traffic along the project area from south of Parker Road to north of Colfax Avenue on the general purpose and express toll lanes was summarized for the optimum toll rates. Additionally, weekday volumes on the express toll lanes by time period are also provided. These volumes at 2010 and 2025 levels are displayed in Figures 3-17 and 3-18, respectively.

Estimated year 2010 traffic at the tolling zone between Alameda and $6^{\text {th }}$ Avenues on the express toll lanes is 30,800 vehicles. This volume represents about 18 percent of the total I-225 traffic demand of 169,600 vehicles at this location. By 2025, weekday express toll lane volumes are estimated to increase to 39,900 vehicles or 22 percent of the total demand of 184,400 vehicles.

Estimated Annual Trips And Gross Toll Revenue - Annual trips and gross toll revenue for Scenario 1 are provided in Table 3-15. The I-225 express toll lanes are estimated to generate $\$ 11.0$ million in gross toll revenue in year 2010, increasing to an estimated $\$ 25.3$ million by the year 2025. The annual number of trips is estimated at 9.0 million in year 2010, growing to 11.5 million by year 2025.
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Table 3-14
Toll Rates By Time Period
I-225 Express Toll Lanes
I-70 to U.S. 83
(2004 dollars)


|  |  |
| :---: | :---: |


| Time Period |
| :--- |
|  |
| AM Shoulder |
| AM Peak |
| AM Shoulder |
| Midday |
| PM Shoulder |
| PM Peak |
| PM Shoulder |



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| :---: | :---: | :---: |
| Existing General <br> Purpose Lanes | AMPK | A.M. Peak (7-8 A.M.) |
|  | AMSH | A.M. Shoulder (6-7 A.M., 8-9 A.M.) |
| Express Lanes | MD | Midday (9 A.M. 3 P.M.) |
| Access <br> To / From Express Lanes Tolling Zones | PMSH | P.M. Shoulder (3-4 P.M., 6-7 P.M.) |
|  | PMPK | P.M. Peak (4-6 P.M.) |
|  | Daily | Daily (6 A.M. - 7 P.M.) |

Note: All volumes shown represent thousands of vehicles.
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Table 3-15

## Annual Traffic And Revenue Estimates

I-225 Express Toll Lanes

| Year | Annual Trips (1) | Annual Revenue (1, 2) |
| :---: | :---: | :---: |
|  | (000) | (000) |
| 2010 | 9,010 | \$11,009 |
| 2011 | 9,160 | 11,638 |
| 2012 | 9,312 | 12,304 |
| 2013 | 9,467 | 13,007 |
| 2014 | 9,624 | 13,751 |
| 2015 | 9,785 | 14,537 |
| 2016 | 9,947 | 15,368 |
| 2017 | 10,113 | 16,247 |
| 2018 | 10,281 | 17,176 |
| 2019 | 10,452 | 18,158 |
| 2020 | 10,626 | 19,196 |
| 2021 | 10,802 | 20,294 |
| 2022 | 10,982 | 21,454 |
| 2023 | 11,165 | 22,680 |
| 2024 | 11,350 | 23,977 |
| 2025 | 11,539 | 25,348 |
| 2026 | 11,712 | 26,362 |
| 2027 | 11,888 | 27,416 |
| 2028 | 12,066 | 28,513 |
| 2029 | 12,247 | 29,654 |
| 2030 | 12,431 | 30,840 |
| 2031 | 12,555 | 31,765 |
| 2032 | 12,681 | 32,718 |
| 2033 | 12,807 | 33,699 |
| 2034 | 12,936 | 34,710 |
| 2035 | 13,065 | 35,752 |
| 2036 | 13,130 | 36,467 |
| 2037 | 13,196 | 37,196 |
| 2038 | 13,262 | 37,940 |
| 2039 | 13,328 | 38,699 |
| 2040 | 13,395 | 39,473 |

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## I-270 EXPRESS TOLL LANES - SCENARIO 1

The I-270 Scenario 1 project spans approximately five miles between I-25 and I-70. I-270 was assumed to have two general purpose and two express toll lanes in each direction. The express toll lanes were assumed to be located in the median of the existing roadway and separated from the general purpose lanes by a concrete barrier.

## Current Traffic Conditions

For the analysis of these scenarios the latest regional traffic model was obtained for the DRCOG. A collection of hourly traffic counts on all I270 ramps within the study corridor were used to create an hourly traffic demand profile for 2002. The highest average weekday volume of 98,500 vehicles on I-270 occurs between U.S. 85 and York Street. Hourly variations along this section show that southbound traffic is highest during the a.m. peak while westbound traffic reaches near capacity levels during the p.m. peak period. The direct connectors to I-70 east carry over 60,000 vehicles on an average weekday.

## Project Access Points And Tolling Concept

The assumed points of express toll lane access and tolling for the I-270 express toll lanes are shown in Figure 3-19. Entry to and exit from the express toll lanes are assumed to be made via four locations. Access was assumed at the following locations:

- To/from the south, north of I-76;
- To/from the south, south of York Street;
- To/from the north, south of U.S. 85; and
- Direct connectors to I-70.

Figure 3-19 also identifies the general location of the tolling zone. Because the project is only about five miles in total length, it was assumed a flat rate structure would be used, where the toll rate would be assessed at the common location to all express lane traffic located between the York Street and U.S. 86 interchanges. All traffic, no matter entering or exiting point, would be assessed the same toll rate, although the rate would vary by time of day and direction of travel.

## Preliminary Traffic and Revenue Estimates

Presented below are the estimated optimum toll rates by time period which were the outcome of toll sensitivity analyses performed for the assumed opening year (2010) and future year (2025). Also presented below are estimates of average weekday traffic for years 2010 and 2025. Volumes shown are by time period and total weekday on the express toll lanes and



I-270 PROPOSED ETL ACCESS POINTS AND TOLLING ZONES

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total weekday on the general purpose lanes. Finally, estimates of the annual number of trips and gross toll revenue are provided.

Toll Rates - A toll sensitivity analysis was conducted for each analysis period in years 2010 and 2025. A flat rate structure as mentioned above was used in which toll rates tested ranged from $\$ 0.25$ to $\$ 5.00$. Shown in Table 3-16 are the optimum passenger car-based toll rates by time period and travel direction. The tolls selected range from $\$ 0.50$ to $\$ 2.50$ in year 2010 and from $\$ 0.75$ to $\$ 4.00$ in 2025, depending on the time period.

Estimated Traffic - Average weekday toll traffic along I-270 on the general purpose and express toll lanes was summarized at the optimum toll rates. Additionally, weekday volumes on the express toll lanes by time period are also provided. These volumes at 2010 and 2025 levels are displayed in Figures 3-20 and 3-21. For year 2010, 25,900 vehicles on an average weekday are estimated to pass through the tolling zone. All traffic using the facility has to pass through this point and, therefore, would have the largest volume along the express toll lanes. This volume represents about 17 percent of the total I-270 demand at this location. At this same location during the a.m. peak hour, the estimated southbound volume is 2,100 vehicles on the two express toll lanes. The northbound p.m. two hour peak period volume is estimated at 4,700 vehicles.

By 2025 at this same location, the weekday express toll lane volume is estimated to increase to 37,500 vehicles or 20 percent of the total demand. The a.m. peak hour southbound traffic on the managed lanes is at its assumed maximum threshold and a high toll rate would be needed to manage demand within the express toll lanes.

Estimated Annual Trips And Gross Toll Revenue - Annual trips and gross toll revenue are provided in Table 3-17. Annual gross toll revenue in year 2010 is estimated to be $\$ 10.9$ million, growing to $\$ 25.8$ million by 2025. The annual number of trips is estimated at 7.1 million in year 2010, growing to 10.6 million by year 2025 .

## C-470 EXPRESS TOLL LANES - SCENARIOS 1, 1A, 2 AND 2A

Four C-470 express toll lane scenarios have been evaluated. The scenarios are as follows:

- Scenarios 1 and 1A - Both scenarios are approximately 14 miles in length, extending from just east of I-25 to Kipling Parkway; and

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Note: All volumes shown represent thousands of vehicles.


Note: All volumes shown represent thousands of vehicles.


Table 3-17
Annual Traffic And Revenue Estimates
I-270 Express Toll Lanes

| Year | Annual Trips (1) | Annual Revenue (1, 2) |
| :---: | :---: | :---: |
|  | (000) | (000) |
| 2010 | 7,123 | \$10,884 |
| 2011 | 7,315 | 11,530 |
| 2012 | 7,512 | 12,214 |
| 2013 | 7,715 | 12,938 |
| 2014 | 7,923 | 13,706 |
| 2015 | 8,136 | 14,519 |
| 2016 | 8,355 | 15,380 |
| 2017 | 8,581 | 16,292 |
| 2018 | 8,812 | 17,259 |
| 2019 | 9,049 | 18,283 |
| 2020 | 9,293 | 19,367 |
| 2021 | 9,544 | 20,516 |
| 2022 | 9,801 | 21,733 |
| 2023 | 10,065 | 23,022 |
| 2024 | 10,336 | 24,388 |
| 2025 | 10,615 | 25,835 |
| 2026 | 10,827 | 26,868 |
| 2027 | 11,044 | 27,943 |
| 2028 | 11,265 | 29,061 |
| 2029 | 11,490 | 30,223 |
| 2030 | 11,720 | 31,432 |
| 2031 | 11,837 | 32,375 |
| 2032 | 11,955 | 33,346 |
| 2033 | 12,075 | 34,347 |
| 2034 | 12,196 | 35,377 |
| 2035 | 12,318 | 36,439 |
| 2036 | 12,379 | 37,167 |
| 2037 | 12,441 | 37,911 |
| 2038 | 12,503 | 38,669 |
| 2039 | 12,566 | 39,442 |
| 2040 | 12,629 | 40,231 |

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- Scenarios 2 and 2A - Both scenarios are approximately 26 miles in length and extend from just east of I-25 to I-70.

Traffic and revenue estimates for Scenarios 1 and 2 were derived from the "base" DRCOG trip tables. Traffic and revenue estimates for Scenarios 1A and 2A were developed using an alternative traffic growth scenario between years 2010 and 2025. These and other project assumptions used in the analysis of C-470 express toll lanes, along with the analytical findings are presented in detail below.

The C-470 Scenarios 1 and 1A projects span approximately 14 miles from just east of I-25, connecting to the terminus of the existing E-470 Tollway, to Kipling Parkway. C-470 was assumed to have two general purpose and two express toll lanes in each direction from I-25 to east of Wadsworth Boulevard, and one express toll lane per direction from east of Wadsworth Boulevard to Kipling Parkway. The express toll lanes are assumed to be located in the median of the existing roadway and separated from the general purpose lanes by a concrete barrier, except for the segment between Kipling Parkway and east of Wadsworth which would be separated by a four foot buffer.

The C-470 Scenarios 2 and 2A project limits are from just east of I-25, connecting to the terminus of the existing E-470 Tollway, to I-70. The project is approximately 26 miles long. Scenario 2 was assumed to have two general purpose and two express toll lanes in each direction along its entire length. (This was assumed for analysis purposes only. The WSA study team recognizes that there are currently six general purpose lanes between Morrison Road and I-70, and that Colorado law does not permit tolling of existing capacity.) The express toll lanes were assumed to be located in the median of the existing roadway and separated from the general purpose lanes by a concrete barrier.

Traffic and revenue estimates for Scenarios 1 and 2 of the C-470 project were developed using the base trip tables from the DRCOG model, after developing new trip distributions to reflect the hypothetical additional capacity on C-470. However, even with the additional capacity, the model showed very low levels of growth in traffic demand, averaging only between 1 and 2 percent per year through 2010 and less than 1 percent per year after 2010.

A detailed independent economic review of this or other corridors was beyond the scope of this feasibility study. Indeed, the extremely low growth rates and travel demand for the C-470 corridor may well be appropriate. However, given the preliminary nature of this feasibility

assessment, an alternative growth scenario was tested, which doubled the rate of projected traffic growth between 2010 and 2025 only. Even with this hypothetical increased level of growth, total growth in corridor demand along the C-470 corridor still would average less than 2 percent per year, subsequent to 2010.

Again, the purpose of this alternative growth scenario was to assess the potential impact on traffic and revenue for the C-470 express toll lanes given a level of future growth which was more consistent with that being seen on other freeways throughout the region. This was a hypothetical alternative, and is not intended to suggest that the base line traffic growth forecasts within the DRCOG model are necessarily inappropriate. These are simply two alternative growth conditions evaluated as part of this preliminary study.

## Current Traffic Conditions

For the analysis of these scenarios the latest regional traffic model was obtained for the DRCOG. A collection of hourly traffic counts on all C470 ramps within the study corridor and at three mainline locations along C-470 were used to create an hourly traffic demand profile for 2003.

The highest traffic volume along the corridor is found between Quebec Street and Yosemite Street, where there are approximately 102,000 vehicles on an average weekday. Traffic levels continue to be near 100,000 vehicles until west of S. Broadway. Between S. Broadway and Wadsworth Boulevard average weekday traffic ranges from 68,000 to 79,000 vehicles. The lowest volumes along the corridor are found between Wadsworth Boulevard and W. Bowles Avenue where average weekday volumes are approximately 55,000 vehicles. Average weekday volumes increase, approaching I-70 to a maximum of 82,500 vehicles north of U.S. 285.

Hourly variation patterns along the mainline sections of C-470 showed that the west section of C-470 has significant directional peaking patterns. There, traffic is heaviest in the morning northbound and southbound during the afternoon peak. The eastern section of the C-470 has significant peaking patterns in both directions during the a.m. and p.m. peak periods with the highest peaks in traffic occurring eastbound during a.m. and westbound during the p.m.

## Project Access Points And Tolling Concept

The assumed points of express toll lane access and tolling for Scenarios 1, 1A, 2 and 2A are shown in Figure 3-22. Entry to and exit from the


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express toll lanes were assumed to be provided via 9 locations in Scenarios 1 and 1A, and 12 locations in Scenarios 2 and 2A.

In Scenarios 1 and 1A access is provided at the following locations:

- To/from the east, east of Kipling Parkway;
- To/from the east, east of Wadsworth Boulevard;
- To/from the east and west, west of Broadway;
- From the east and west, west of University Boulevard;
- Direct connections to/from the east at Colorado Boulevard;
- Direct connections to/from the west at Quebec Street;
- To/from the west at Yosemite Street;
- To/from the west with I-25; and
- To/from the west, east of I-25.

Except for several additional express toll lane access points which will be described below, eight of the nine access points identified above for Scenarios 1 and 1A are identical to those assumed for Scenarios 2 and 2A.

In Scenarios 2 and 2A, the access which was to/from the east, only, east of Wadsworth Boulevard becomes full directional access. New access points are assumed to be provided at the following locations:

- To/from the north and south, north of Bowles Avenue;
- To/from the north and south, north of U.S. 285; and
- To/from the south, south of I-70.

Figure 3-22 also identifies the general location of tolling zones for each scenario. There are six tolling zones identified in Scenarios 1 and 1A and nine in Scenario 2 and 2A. Under Scenarios 1 and 1A, the far west tolling zone would cover the distance traveled from Wadsworth Boulevard to the Santa Fe Drive access, the middle tolling zone would cover the distance traveled from the Santa Fe Drive access to University Boulevard and the last tolling zone would cover the distance from the University Boulevard access to I-25.

In addition to the three tolling zones described above, Scenarios 2 and 2A, include three additional tolling zones. The first additional tolling zone covers the distance traveled between Kendall Boulevard and Quincy Avenue, the second, the distance traveled between Bowles Avenue and Morrison Road, and the third covering the distance between U.S. 285 and the end of project's north end.

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## Preliminary Traffic and Revenue Estimates

Presented below are the estimated optimum toll rates by time period which were the outcome of toll sensitivity analyses performed for the assumed opening year (2010) and future year (2025). Also presented below are estimates of average weekday traffic for years 2010 and 2025. Volumes shown are by time period and total weekday on the express toll lanes and total weekday on the general purpose lanes. Finally, estimates of the annual number of trips and gross toll revenue for each scenario are provided.

Toll Rates - A toll sensitivity analysis was conducted for each analysis period in years 2010 and 2025 for the four scenarios. A per-mile rate structure was assumed for all scenarios. Per-mile toll rates tested ranged from $\$ 0.050$ to $\$ 0.450$. Shown in Tables 3-18 through 3-21 are the optimum passenger car-based per-mile toll rates by time period and travel direction for Scenarios 1, 1A, 2 and 2A, respectively. Also presented is the toll for a full-length trip by time period and direction.

The full-length tolls in Scenarios 1 and 1A range from $\$ 1.00$ to $\$ 4.75$ in 2010. By 2025, the full-length tolls in Scenario 1 increase during the peak periods, ranging from $\$ 1.00$ to $\$ 6.00$. Due to the increased traffic growth assumption for year 2025 in Scenario 1A, toll rates increase over Scenario 1 due primarily to increases in congestion in the general purpose lanes. Year 2025 tolls in Scenario 1A range from $\$ 1.60$ to $\$ 6.75$.

The tolls in Scenarios 2 and 2A for a 26 mile full-length trip range from $\$ 1.25$ to $\$ 7.75$ in year 2010. By 2025, the full-length tolls in Scenario 2 increase during the peak periods, ranging from $\$ 1.25$ to $\$ 11.00$. Due to the increased traffic growth assumption for year 2025 in Scenario 2A, some toll rates increase over Scenario 2 due primarily to increases in congestion in the general purpose lanes. In practice, due to the length and orientation of C-470 from I-70 to I-25, it is likely that there would be very few "through trips" on the full, express toll lane project.

Estimated Traffic - Average toll weekday traffic along C-470 on the general purpose and express toll lanes was summarized for each scenario at the optimum toll rates. Additionally, weekday volumes on the express toll lanes by time period are also provided. These volumes at 2010 and 2025 levels are displayed in Figure 3-23 for Scenario 1, and Figure 3-24 for Scenario 1A.

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Time Period

AM Shoulder
AM Peak
AM Shoulder
Midday
PM Shoulder
PM Peak
PM Shoulder

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$$
\begin{aligned}
& \begin{array}{c}
\text { Table 3-20 } \\
\text { Toll Rates By Time Period } \\
\text { C-470 Express Toll Lanes } \\
\text { Scenario 2: I-25 to I-70 } \\
\text { (2004 dollars) }
\end{array}
\end{aligned}
$$

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\begin{aligned}
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In Scenario 1, the peak load point on the express toll lanes in year 2010 is 29,200 vehicles at the tolling zone west of Quebec Street. This volume represents about 23 percent of the total C-470 demand at this location. By 2025 at this same location, the weekday express toll lane volume is estimated to increase to over 35,000 vehicles or 24 percent of the total demand.

The only differences in estimated traffic volumes between Scenarios 1 and 1A occur in the year 2025. This was the result of growth adjustments made to the micro-model time period trip tables. Existing trip tables produced traffic growth of less than one percent per year. Based on historical growth trends, this level of annual average growth was considered extremely conservative. Therefore, the growth between years 2010 and 2025 within the micro-model trip tables were adjusted, reflecting an average annual growth rate of 1.5 percent per year. Based on this revised growth, year 2025 express lane traffic at the tolling zone west of Quebec Street increased to over 41,000, while total demand in both the general purpose and express toll lanes increased from 144,800 in Scenario 1 to 157,400 in Scenario 1A.

Figures 3-25 and 3-26 show traffic estimates for Scenario 2. In Scenario 2, the peak load point on the express toll lanes in year 2010 is 28,600 vehicles at the tolling zone west of Quebec Street. This volume represents about 22 percent of the total C-470 demand at this location. At the other tolling zones between I-25 and Wadsworth Boulevard, express lane demand ranges between 11,000 and 22,000 vehicles. Lower volumes are experienced at the three tolling zones west of Wadsworth Boulevard, where estimated daily volumes range from approximately 6,500 to 15,000 vehicles. By 2025, daily express lane volumes are expected to increase to over 37,000 vehicles at the location west of Quebec Street or 25 percent of the total traffic demand of 146,500 vehicles in both general purpose and express toll lanes. This volume is almost 15,000 vehicles per day higher than in 2010.

As with Scenarios 1 and 1A, the only differences in estimated traffic volumes between Scenarios 2 and 2A occur in the year 2025. As shown in Figure 3-27, based on this revised growth, year 2025 express lane traffic at the tolling zone west of Quebec Street increased to 43,600 vehicles, while total demand in both the general purpose and express toll lanes increased from 146,500 vehicles in Scenario 2 to 156,500 vehicles in Scenario 2A. The Scenario 2A 2025 total demand at this tolling zone is 1.15 percent per year higher than volumes estimated for year 2010.


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2010 ESTIMATED WEEKDAY TRAFFIC - C-470 SCENARIOS 2 AND 2A: l-25 TO I-70


| LEGEND |  |  |
| :--- | :--- | :--- |
| 0.0 Existing General | AMPK A.M. Peak (7-8 A.M.) |  |
| 0.0 | Purpose Lanes | AMSH |
| Express Lanes | A.M. Shoulder (6-7 A.M., 8-9 A.M.) | Midday (9 A.M. - 3 P.M.) |
| Access | PMSH | P.M. Shoulder (3-4 P.M., 6-7 P.M.) |
| To / From | PMPK | P.M. Peak (4-6 P.M.) |
| Express Lanes | Daily | Daily (6 A.M. - 7 P.M.) |
| Tolling Zones |  |  |

Note: All volumes shown represent thousands of vehicles.


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2025 ESTIMATED WEEKDAY TRAFFIC - C-470 SCENARIO 2: I-25 TO I-70
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2025 ESTIMATED WEEKDAY TRAFFIC - C-470 SCENARIO 2A: I-25 TO I-70


Estimated Annual Trips And Gross Toll Revenue - Annual trips and gross toll revenue for Scenarios 1, 1A 2, and 2A are provided in Tables 322 through 3-25, respectively. Scenario 1 produces an estimated $\$ 14.5$ million in gross toll revenue in year 2010 and grows to an estimated \$24.1 million by year 2025. The annual number of trips is estimated at 10.6 million in year 2010, growing to 13.0 million by year 2025.

Scenario 1A produces an estimated $\$ 14.5$ million in gross toll revenue in year 2010 and grows to an estimated $\$ 37.1$ million by year 2025. The annual number of trips is estimated at 10.6 million in year 2010, growing to 16.6 million by year 2025 .

Gross toll revenue produced by Scenario 2 is estimated at $\$ 22.3$ million in year 2010 and $\$ 36.6$ million in year 2025. The annul number of trips in the express toll lanes rises from 14.5 million in year 2010 to 16.5 million in year 2025.

Scenario 2A is estimated to produce $\$ 22.3$ million in gross toll revenue in 2010, growing to $\$ 56.9$ million by 2025 . The annual number of trips for 2010 is estimated at 14.5 million and is estimated to increase to 21.5 million by year 2025

## NORTHWEST CORRIDOR TOLL ROAD - SCENARIOS 1 AND 2

Two scenarios were considered for this corridor. Scenario 1 consisted of a new toll road between U.S. 36 and C-470. The project of approximately 24 miles in length was assumed to be a four-lane roadway on new alignment. Scenario 2 was assumed to follow the same alignment as Scenario 1, but the tolled section would only extend approximately 14 miles from S.H. 128 to S.H. 58. The detailed assumptions for these scenarios, along with the analytical findings, are presented below.

The Northwest Corridor Scenario 1 project was assumed to consist of developing a new roadway corridor between U.S. 36 and C-470, connecting it to the existing Northwest Parkway Tollway and completing the outer beltway around Denver. The new corridor was assumed to be approximately 24 miles long and include a four-lane roadway on new alignment. New interchanges were assumed at nine locations along the corridor at major interstate, highway and arterial crossings.
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Table 3-22
Annual Traffic And Revenue Estimates
C-470 Express Toll Lanes
Scenario 1 : I-25 to Kipling Parkway

| Year | Annual Trips (1) | Annual <br> Revenue (1) |
| :---: | :---: | :---: |
|  | (000) | (000) |
| 2010 | 10,551 | \$14,491 |
| 2011 | 10,699 | 14,992 |
| 2012 | 10,850 | 15,511 |
| 2013 | 11,003 | 16,047 |
| 2014 | 11,157 | 16,602 |
| 2015 | 11,314 | 17,177 |
| 2016 | 11,474 | 17,771 |
| 2017 | 11,635 | 18,385 |
| 2018 | 11,799 | 19,021 |
| 2019 | 11,965 | 19,679 |
| 2020 | 12,133 | 20,360 |
| 2021 | 12,304 | 21,064 |
| 2022 | 12,477 | 21,793 |
| 2023 | 12,652 | 22,546 |
| 2024 | 12,830 | 23,326 |
| 2025 | 13,011 | 24,133 |
| 2026 | 13,141 | 24,857 |
| 2027 | 13,273 | 25,603 |
| 2028 | 13,405 | 26,371 |
| 2029 | 13,539 | 27,162 |
| 2030 | 13,675 | 27,977 |
| 2031 | 13,811 | 28,676 |
| 2032 | 13,950 | 29,393 |
| 2033 | 14,089 | 30,128 |
| 2034 | 14,230 | 30,881 |
| 2035 | 14,372 | 31,653 |
| 2036 | 14,516 | 32,286 |
| 2037 | 14,661 | 32,932 |
| 2038 | 14,808 | 33,591 |
| 2039 | 14,956 | 34,262 |
| 2040 | 15,105 | 34,948 |



Table 3-23
Annual Traffic And Revenue Estimates
C-470 Express Toll Lanes
Scenario 1A : I-25 to Kipling Parkway
$\left.\begin{array}{lcc}\text { Year } & \begin{array}{c}\text { Annual } \\ \text { Trips (1) }\end{array} & \end{array} \begin{array}{c}\text { Annual } \\ \text { Revenue (1) }\end{array}\right]$
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Table 3-24
Annual Traffic And Revenue Estimates C-470 Express Toll Lanes
Scenario 2 : I-25 to I-70
$\left.\begin{array}{lcc}\text { Year } & \begin{array}{c}\text { Annual } \\ \text { Trips (1) }\end{array} & \end{array} \begin{array}{c}\text { Annual } \\ \text { Revenue (1) }\end{array}\right]$.
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Table 3-25
Annual Traffic And Revenue Estimates
C-470 Express Toll Lanes
Scenario 2A : I-25 to I-70
$\left.\begin{array}{ccc}\text { Year } & \begin{array}{c}\text { Annual } \\ \text { Trips (1) }\end{array} & \end{array} \begin{array}{c}\text { Annual } \\ \text { Revenue (1) }\end{array}\right]$


The Northwest Corridor Scenario 2 project was assumed to follow the same alignment as Scenario 1, however, the tolled section of the corridor was assumed to extend from S.H. 128 to S.H 58. This new corridor was assumed to be approximately 14 miles long and include a four-lane roadway on new alignment. New interchanges were assumed at five locations along the corridor at major highway and arterial crossings. Since tolls were assumed to be levied along the S.H. 128 to S.H. 58 segment only, tolling of existing Highway 93 capacity would not occur.

## Current Traffic Conditions

A major north-south facility in the immediate project corridor is S.H. 93. This highway carries year 2004 average weekday traffic (AWDT) volumes in the range of 20,000 to 25,000 vehicles on segments from S.H. 128 in the north to S.H. 58 in the south. South of S.H. 58, the road carries the U.S. 6 designation through the heart of Golden, where the (AWDT) falls in the range of 30,000 to 50,000 vehicles.

Other north-south roads in the project corridor which have sizeable average weekday traffic volumes include McIntyre Street, Indiana Street and Wadsworth Boulevard. McIntyre Street, closer to the corridor's south end, carries AWDT volumes in the range of 10,000 to 15,000 vehicles. Indiana Street, generally in the north-central portion of the corridor, carries AWDT volumes in the range of 12,000 between S.H. 128 and S.H. 72. South of S.H. 72 volumes increase to approximately 19,000 vehicles. Along Wadsworth Boulevard at the far east end of the project corridor, AWDT falls in the 30,000 to 35,000 vehicles range between U.S. 36 and $88^{\text {th }}$ Avenue. Between $88^{\text {th }}$ Avenue and I-70 through Arvada, AWDT volumes are over 50,000 vehicles.

## Project Access Points And Tolling Concept

Figure 3-28, presents the assumed interchange access locations for the project. Access to the Scenario 1 project was assumed to be provided from all major highway and arterial crossings between the Northwest Parkway to the north and I-70 to the south. Apart from interchanges at the project termini, the interchanges include U.S. 36, S.H. 128, Vauxmont Road, S.H. 72, $64{ }^{\text {th }}$ Parkway, S.H. 58 and $6^{\text {th }}$ Avenue. The electronic toll collection concept is comprised of a toll zone between each interchange as illustrated in Figure 3-29.

Access to the Scenario 2 project was also assumed to be provided from all major highway and arterial crossings. However, for this shorter toll project, access to the tolled facility was assumed to be provided at S.H. 128, Vauxmont Road, S.H. 72, $64^{\text {th }}$ Parkway and S.H. 58. The electronic toll collection concept for Scenario 2 is also illustrated in Figure 3-29.


SCENARIO 1: U.S. 36 TO I-70 / C-470


SCENARIO 2: S.H 128 TO S.H. 58


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## NORTHWEST CORRIDOR PROPOSED



Note：All volumes shown are in thousands．

## SCENARIO 1：U．S． 36 TO I－70／C－470



SCENARIO 2：S．H 128 TO S．H． 58


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## Preliminary Traffic And Revenue Estimates

Presented below are the estimated optimum toll rates by time period which were the outcome of toll sensitivity analyses performed for the assumed opening year (2010) and future year (2025). Also presented below are estimates of average daily traffic for years 2010 and 2025. Finally, estimates of the annual number of trips and gross toll revenue for each scenario are provided.

Toll Rates - The toll rates tested for both Scenarios 1 and 2 ranged from approximately $\$ 0.05$ to $\$ 0.25$ per mile, at increments of $\$ 0.05$. For every toll rate tested a unique revenue yield was produced, from which a toll sensitivity curve was developed. Based on review of toll sensitivity curves for years 2010, a toll rate of $\$ 0.15$ per mile was selected for both scenarios. For the year 2025, the optimum toll equated to a rate of approximately $\$ 0.20$ per mile.

Estimated Traffic - Figure 3-29 presents estimates of average daily traffic for the years 2010 and 2025 for both Scenarios 1 and 2. For the 24 mile Scenario 1 project, opening year average daily traffic ranges from an estimated 10,800 vehicles between the I-70 and $6^{\text {th }}$ Avenue Interchanges to almost 23,000 vehicles between the S.H. 128 and Vauxmont Road Interchanges. By 2025, volumes at these same locations increase to 12,800 and 29,000, respectively.

For the 14 mile Scenario 2 project, opening year average daily traffic ranges from an estimated 24,500 between the Vauxmont Road and S.H. 72 Interchanges to almost 37,800 between the $64^{\text {th }}$ Parkway and S.H. 58 Interchanges. By 2025, volumes at these same locations increase to 32,500 and 43,200, respectively.

Estimated Annual Trips and Gross Toll Revenue - Table 3-26 presents the traffic and revenue summary for Scenario 1. For opening year 2010, the number of annual trips is estimated to be 17.0 million, producing annual toll revenue of $\$ 34.7$ million. By 2025, the annual trips on the proposed facility increase to 20.6 million, generating annual revenue of $\$ 51.3$ million.

Future year trips and revenue for Scenario 2 are shown in Table 3-27. In the opening year, the annual number of trips is estimated to be 20.3 million, producing approximately $\$ 24.5$ million in revenue. By 2025, the number of annual trips is estimated to increase to 24.1 million, while toll revenues are estimated to rise to $\$ 30.2$ million.
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Table 3-26
Annual Traffic And Revenue Estimates
Northwest Corridor
Scenario 1 : U.S. 36 to S.H. 6

| Year | Annual Trips (1) |  |
| :---: | :---: | :---: |
|  | $(000)$ | Annual Revenue (1, 2) |
| 2010 |  | $(000)$ |
| 2011 | 16,971 |  |
| 2012 | 17,192 | 35,663 |
| 2013 | 17,416 | 35,579 |
| 2014 | 17,643 | 36,519 |
| 2015 | 17,873 | 37,484 |
| 2016 | 18,106 | 38,474 |
| 2017 | 18,342 | 39,491 |
| 2018 | 18,581 | 40,534 |
| 2019 | 18,823 | 41,605 |
| 2020 | 19,068 | 42,704 |
| 2021 | 19,316 | 43,832 |
| 2022 | 19,568 | 44,990 |
| 2023 | 19,823 | 46,179 |
| 2024 | 20,081 | 47,399 |
| 2025 | 20,343 | 48,651 |
| 2026 | 20,608 | 49,936 |
| 2027 | 20,814 | 51,256 |
| 2028 | 21,022 | 52,537 |
| 2029 | 21,232 | 53,850 |
| 2030 | 21,445 | 55,197 |
| 2031 | 21,659 | 56,577 |
| 2032 | 21,876 | 57,991 |
| 2033 | 22,095 | 59,151 |
| 2034 | 22,316 | 60,334 |
| 2035 | 22,539 | 61,541 |
| 2036 | 22,764 | 62,771 |
| 2037 | 22,878 | 64,027 |
| 2038 | 22,992 | 64,667 |
| 2039 | 23,107 | 65,314 |
| 2040 | 23,223 | 66,967 |
|  | 23,339 | 67,293 |
|  |  |  |

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Table 3-27
Annual Traffic And Revenue Estimates
Northwest Corridor
Scenario 2 : S.H. 128 to S.H. 58

| Year | Annual Trips (1) | Annual Revenue (1, 2) |
| :---: | :---: | :---: |
|  | (000) | (000) |
| 2010 | 20,320 | \$24,459 |
| 2011 | 20,553 | 24,806 |
| 2012 | 20,789 | 25,158 |
| 2013 | 21,028 | 25,516 |
| 2014 | 21,270 | 25,878 |
| 2015 | 21,514 | 26,245 |
| 2016 | 21,761 | 26,618 |
| 2017 | 22,011 | 26,996 |
| 2018 | 22,264 | 27,379 |
| 2019 | 22,519 | 27,768 |
| 2020 | 22,778 | 28,162 |
| 2021 | 23,039 | 28,562 |
| 2022 | 23,304 | 28,968 |
| 2023 | 23,572 | 29,379 |
| 2024 | 23,842 | 29,796 |
| 2025 | 24,116 | 30,219 |
| 2026 | 24,357 | 30,642 |
| 2027 | 24,601 | 31,071 |
| 2028 | 24,847 | 31,506 |
| 2029 | 25,095 | 31,947 |
| 2030 | 25,346 | 32,394 |
| 2031 | 25,600 | 32,394 |
| 2032 | 25,856 | 32,718 |
| 2033 | 26,114 | 33,046 |
| 2034 | 26,375 | 33,376 |
| 2035 | 26,639 | 33,710 |
| 2036 | 26,772 | 34,047 |
| 2037 | 26,906 | 34,387 |
| 2038 | 27,041 | 34,731 |
| 2039 | 27,176 | 35,078 |
| 2040 | 27,312 | 35,429 |

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## DISCLAIMER

Current professional practices and procedures were used in the development of these findings. However, there is considerable uncertainty inherent in future traffic and revenue forecasts for any toll facility. There may sometimes be differences between forecasted and actual results caused by events and circumstances beyond the control of the forecasters. These differences could be material. Also, it should be recognized that traffic and revenue forecasts in this document are intended to reflect the overall estimated long-term trend. Actual experience in any given year may vary due to economic conditions and other factors.


This chapter describes the traffic and revenue study of projects generally outside of the Denver metropolitan area. These were located in various areas including Fort Collins, Colorado Springs, the I-70 Mountain Corridor, and the eastern Front Range. In general, most of the projects were studied as new toll roads limited to electronic toll collection only. The one exception was a study of the I-70 Mountain Corridor, in which one of the alternatives included a study of reversible express toll lanes limited to electronic tolling only.

In addition to the development traffic and revenue estimates, existing daily traffic volume data is presented, along with the proposed project interchange locations and tolling concepts.

## PROJECT DESCRIPTIONS

The study consisted of analyzing five corridors: U.S. 287-I-25 Connector; Front Range Toll Road; Powers Boulevard; Banning Lewis Parkway; and the I-70 Mountain Corridor. Table 4-1 presents the list of project corridors and the 14 alternative project scenarios that were studied.

## ANALYSIS METHODOLOGY

Presented below is a brief discussion of the general methodology used to develop the traffic and revenue forecasts, along with an overview of the toll collection system for each project.


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## Traffic Modeling

WSA utilized its proprietary toll diversion traffic model to analyze the potential traffic and revenue impacts to the proposed toll facilities. The toll diversion model estimates the market share of toll facilities versus alternative routes. The model considers toll costs, values of time, vehicle operating costs, highway capacity, travel time and distance parameters, etc.

Traffic model data sets were obtained from the relevant planning jurisdictions or study teams for each of the identified project corridors. These included highway networks, socioeconomic data files and trip tables from the North Front Range Council of Governments (NFRCOG), from the Denver Regional Council of Governments (DRCOG), from the Pikes Peak Area Council of Governments (PPACOG), the Pueblo Metropolitan Planning Organization (PMPO) and the I-70 PEIS Travel Demand Model.

The networks and trip tables were modified to a Tranplan traffic model format. For each of the models, WSA attempted to calibrate model traffic assignments to actual traffic counts. During this process, travel speeds were adjusted and zone centroid connectors were positioned to best represent traffic loading into the network. As part of this process, traffic screenlines were developed to capture major east-west or north-south movements including the project and major alternative routes. Once, satisfactory calibration was reached, future year traffic assignments were conducted with the proposed improvements and tolling concepts. In general, traffic models were prepared for the opening year 2010 and other future years. For each of the scenarios, a series of increasing toll rates were tested to establish toll sensitivity curves.

For certain corridors and scenarios additional work was performed or methodologies adopted, these are described below:

- The U.S. 287-I-25 Connector Toll Road was covered in the northern periphery of the NFRCOG model. It was observed that U.S. 287 and I25 links in the immediate study area were represented as external links into the model. While it would be possible to assess the impact of the new connector to existing east-west routes, such as S.H. 14; it would not be possible to model the potential route switching between U.S. 287 and I-25 for traffic orientated to and from Laramie, Wyoming. As such, a manual toll diversion technique was employed that attempted to estimate total demand on I-25 to and from Laramie and the travel demand to the new toll road connector. This was based on travel time costs, operating costs and toll charges comparisons for the toll and non-toll route and a resulting market share between both routes.

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- The Front Range Toll Road extends a distance of approximately 194miles. In order to fully model the entire road, several models were combined. This included the NFRCOG, DRCOG, PPACOG and the PMPO traffic models. The process involved combining each of the highway networks into a common system, and recoding of the individual traffic analysis zones. Where necessary, highway network links were extended to provide connectivity to each of the MPO networks. An external trip table was developed that was merged with the internal trip tables of the four MPO's. The internal and external components of these trip tables were separated. The internal trips were retained and the external trips were used a basis for the development of interregional trips.
- The Powers Boulevard and Banning Lewis projects were studied using the PPACOG 2025 model. New 2030 socioeconomic information was also provided by PPACOG. This new dataset incorporated a portion of the proposed Banning Lewis Development. Prior socioeconomic forecasts were compared with the new socioeconomic forecasts for select zones in the Banning Lewis influence area. Based on these comparisons, some adjustments were made to the trip tables, via a fratar process. It was noted that the 2020 PPACOG model was very coarse in the Banning Lewis area and would require considerable refinement to better reflect the proposed land uses in more detailed studies in the future.
- For the I-70 Mountain Corridor, Scenario 1 was studied as reversible express toll lanes using a similar methodology described in Chapter 3. Existing traffic profiles were developed for a typical summer weekday, Saturday and Sunday. For each of these days, detailed hourly traffic profiles were prepared by four time periods representing the AM, PM, midday and nightime conditions. These were consistent with the trip tables time periods received from the I-70 PEIS Mountain Corridor Study. Once these profiles were developed, the base year 2000 trip tables were adjusted to better represent the actual observed profiles. Future year trip tables were prepared by applying the difference in the synthesized base year and future year trips to the calibrated/adjusted base year trip table.


## Toll Collection System Considerations

With the exception of one scenario for the I-70 Mountain Corridor, all of the potential toll facility projects evaluated outside of the Denver area would generally involve construction of new toll facilities, in which all vehicles using the facilities would be subjected to a toll. With the exception of another scenario on I-70, all of these new toll facilities were

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assumed to feature open road tolling, i.e., fully electronic tolling without the option to pay cash.

A detailed description of toll collection concepts applicable to the various potential types of projects was included in Chapter 3. The one exception to the fully electronic toll assumptions was an option for I-70 in which tolls would be imposed on all travelers on I-70, possibly at each of the tunnels undergoing major expansion. In this case, toll collection was assumed to be limited to one travel direction only at each of the tunnels. Given the unique nature of travel in the I-70 Mountain Corridor, however, under a scenario where all traffic would be subject to tolls, it was assumed that both electronic and cash collection facilities would be made available.

## PROPOSED U.S. 287-I-25 CONNECTOR

One scenario was considered for the corridor. The proposed scenario would build a new four lane toll road connecting U.S 287 (Livermore) to I-25.

The project corridor is located just north of the City of Fort Collins, as shown in Figure 4-1. It would provide a new high-speed east-west connector route between I-25 and U.S. 287, a distance of approximately 12 miles. The proposed route would provide for two lanes in both directions with assumed direct full connections at I-25 and U.S. 287. One full directional interchange was assumed to be provided in the vicinity of County Road 15/17. The project would serve as an alternative route for through traffic (without a Fort Collins destination), particularly trucks, which are presently exiting I-25 at S.H. 14 and traveling within the City limits to connect to U.S. 287, traveling north and vice-versa. The U.S. 287 provides a shorter route in terms of distance, when traveling from Fort Collins to Laramie, Wyoming. The alternative route is to travel north on I25 and then west on I-80.

## Current Traffic Conditions

Daily traffic volumes approaching 33,000 vehicles per day (vpd) are observed on I-25, south of the S.H. 14 Mulberry Interchange. North of the S.H. 14 Interchange, traffic volumes decline to around 20,000 vpd. Outside of the Fort Collins area, traffic volumes along I-25 continue to fall off, with traffic levels in the range of 13,000 to $16,500 \mathrm{vpd}$.

Along U.S. 287, traffic volumes ranging from 8,000 to 13,500 are seen between S.H. 14 (North) and S.H. 14 (South Fort Collins). North of


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S.H.14, traffic volumes decline, and range between 4,200 and 5,700 vpd, with trucks representing approximately 30.0 percent of the total.
S.H. 14, Mulberry, the primary east-west route, traffic is currently using to travel between I-25 and U.S. 287 shows traffic volumes in the range of 25,000 to 30,000 , with trucks representing 15.0 percent of the total.

Other east-west routes, north of Fort Collins, which connect I-25 to U.S. 287 carry minor traffic generally less than $3,000 \mathrm{vpd}$. In general, the routes located furthest north show the lowest traffic volumes.

## Project Access Points and Tolling Concept

The new toll road was assumed to provide full directional access at U.S. 287 , CR $15 / 17$, and I- 25 . Tolls were assumed to be collected electronically based on a total of two toll zones located on the mainline segments. These are shown in Figure 4-2.

## Preliminary Traffic and Revenue Estimates

Toll Rates - A series of incremental toll rates were assumed and tested for the Project ranging from $\$ 0.05$ to $\$ 0.25$ per-mile for passenger car tolls, with higher rates for commercial vehicles. Each of the toll rates tested produced a unique revenue yield which formed the basis for establishing a toll elasticity curve. Based on review of the toll sensitivity curve, $\$ 0.15$ per-mile passenger car toll rate was selected. The through rate for a passenger car and heavy truck would be approximately $\$ 1.80$ and $\$ 5.40$, respectively.

Estimated Traffic - The estimated traffic for 2010 and 2020 is also presented in Figure 4-2. This shows opening year traffic in the range of 2,000 to 3,000 vpd. By year 2025, traffic increases to 3,000 to 3,500 vpd. Trucks would represent approximately 30.0 percent of the total traffic.

Estimated Annual Trips and Gross Toll Revenue - Review of Table 4-2 shows that for opening year, 2010, the estimated annual trips would be 733,000 with gross toll revenues of approximately $\$ 1.9$ million. By 2025, estimated annual trips would increase to 922,000 representing an increase of approximately 25.0 percent. For the same year, gross toll revenues are estimated to increase to $\$ 2.9$ million. By 2040, the number of annual trips is shown to be 1.1 million, producing approximately $\$ 3.4$ million in revenues.


Note: All volumes shown are in thousands


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Table 4-2
Annual Traffic And Revenue Estimates
US 287-I-25 Connector Scenario 1

| Year | Annual <br> Trips (1) (000) | Annual <br> Revenue (1) |  |
| :---: | :---: | :---: | :---: |
|  |  |  |  |
| 2010 | 734 | \$ | 1,995 |
| 2011 | 746 |  | 2,054 |
| 2012 | 759 |  | 2,113 |
| 2013 | 771 |  | 2,172 |
| 2014 | 784 |  | 2,230 |
| 2015 | 797 |  | 2,289 |
| 2016 | 809 |  | 2,348 |
| 2017 | 822 |  | 2,407 |
| 2018 | 834 |  | 2,466 |
| 2019 | 847 |  | 2,524 |
| 2020 | 859 |  | 2,583 |
| 2021 | 872 |  | 2,642 |
| 2022 | 884 |  | 2,701 |
| 2023 | 897 |  | 2,759 |
| 2024 | 909 |  | 2,818 |
| 2025 | 922 |  | 2,877 |
| 2026 | 940 |  | 2,932 |
| 2027 | 957 |  | 2,986 |
| 2028 | 974 |  | 3,038 |
| 2029 | 990 |  | 3,087 |
| 2030 | 1,005 |  | 3,135 |
| 2031 | 1,019 |  | 3,180 |
| 2032 | 1,033 |  | 3,222 |
| 2033 | 1,045 |  | 3,262 |
| 2034 | 1,057 |  | 3,299 |
| 2035 | 1,068 |  | 3,333 |
| 2036 | 1,078 |  | 3,364 |
| 2037 | 1,087 |  | 3,392 |
| 2038 | 1,095 |  | 3,417 |
| 2039 | 1,102 |  | 3,438 |
| 2040 | 1,108 |  | 3,457 |

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## PROPOSED FRONT RANGE TOLL ROAD

Two scenarios were considered for this corridor. These included:

- Scenario 1 assumes a new four lane toll road from I-25 N (at Fort Collins) to I-25S (south of Pueblo); and
- Scenario 2 assumes a new four lane toll road from I-25 N (at Fort Collins) to I-25S (north of Pueblo).

The proposed Front Range Toll Road corridor extends 194 miles along the Front Range of the Rocky Mountains. As shown in Figure 4-3, it traverses seven counties with East Central Colorado, including Larimer, Weld, Adams, Arapahoe, Elbert, El Paso, and Pueblo Counties. Two alternative alignments were studied for the Front Range Toll Road. Each of the two scenarios had a northern terminus at I-25, north of Fort Collins at the Wellington Interchange. For the southern terminus, Scenario 1 had an interchange with I-25 south of the St. Charles River, south of Pueblo; whereas Scenario 2 would have an interchange with I-25 north of Pueblo.

The proposed Front Range Toll Road was studied as a four-lane controlled access, tolled highway. Scenarios 1 and 2 were assumed to have 12 and 11 interchanges intersecting with the major routes across the corridor. The total length for Alternative 1 was approximately 194 miles. For Scenario 2, the southerly termini would be located north of Pueblo, eliminating an interchange at U.S. 50, with a total length of approximately 169-miles.

## Current Traffic Conditions

The main routes the proposed North Front Range Toll Road would be competing against would be I-25. Starting from the north end, I-25 in the Fort Collins area currently services daily traffic in the range of 20,000 to 50,000 vpd. In the Denver metro area, I-25 approaches an ADT of approximately 200,000 in the vicinity of the I-70 Interchange. South of the Denver metro area traffic volumes decline to the range of 50,000 to 60,000 vpd. In the Colorado Springs area, I-25 traffic volumes range from 30,000 to $75,000 \mathrm{vpd}$. In the southern most metro area, Pueblo, I-25 traffic volumes range from 40,000 to 70,000 vpd.

## Project Access Points and Tolling Concept

For Scenario 1, the new toll road was assumed to provide full directional access at 12 interchanges. These included interchanges at I-25; U.S. 85; U.S. 34; I-76; DIA; I-70; S.R. 86; U.S. 24; U.S. 94; U.S. 50; and I-25. For Scenario 2, the Project was assumed to terminate short of Pueblo, thus


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eliminating the U.S. 50 Interchange from the prior list of interchanges. The new toll road was assumed to be limited to ETC users only, providing a toll zone for every mainline segment between each of the interchanges. Figure 4-4 presents the assumed location of interchanges and toll zones for Scenarios 1 and 2.

## Preliminary Traffic and Revenue Estimates

Toll Rates - A series of incremental toll rates were tested for the Project ranging from $\$ 0.05$ to $\$ 0.20$ per-mile for passenger car tolls, with higher rates for commercial vehicles. Each of the toll rates tested produced a unique revenue yield which formed the basis for establishing a toll elasticity curve. Based on review of the toll sensitivity curves, $\$ 0.10$ permile passenger car toll rate was assumed. This would be equivalent to a through trip toll of about $\$ 20.00$ for Scenario 1 and about $\$ 17.00$ for Scenario 2.

Estimated Traffic - The estimated traffic for 2010 and 2025 is presented in Figures 4-5 and 4-6 for Scenarios 1 and 2. For Scenario 1, opening year traffic shows traffic volumes in the range of 2,400 to 14,200 vpd. The highest volumes are seen in the Denver metro area, in particular the mainline segment from I-70 to S.H. 86. By 2025, there is modest growth with traffic volumes ranging from 2,600 to 22,000 vpd. Scenario 2 shows similar traffic volumes to that observed in Scenario 1.

Estimated Annual Trips and Gross Toll Revenue - Annual trips and toll revenue are presented in Tables 4-3 and 4-4 for Scenarios 1 and 2, respectively. For Scenario 1, the number in annual trips in estimated be 10.0 million for the opening year. Annual toll revenue is estimated to be $\$ 81.0$ million for the same year. By 2025, the annual number of trips is estimated to increase to 15.3 million, with annual toll revenues reaching approximately $\$ 123.0$ million. The last year shown, 2040, shows the annual number of trips increasing to 19.2 million, producing an estimated $\$ 155.1$ million in toll revenue.

For Scenario 2, the traffic and revenues produced are slightly less than shown for Scenario 1 due to the shorter project distance. For opening year, the project is estimated to serve approximately 9.9 million annual trips, which produce approximately $\$ 76.3$ million in toll revenue. In 2025, the number of annual trips is estimated to increase to 15.2 million, representing an increase of 50.0 percent with toll revenues reaching $\$ 117.8$ million. By 2040, the annual number of trips is shown to be 19.2 million with toll revenues rising to $\$ 149.1$ million.


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## LEGEND



Note: All volumes shown are in thousands


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ESTIMATED YEAR 2010 AND 2025 AVERAGE DAILY TRAFFIC FRONT RANGE SCENARIO 1: I-25N (FORT COLLINS) TO I-25S (SOUTH OF PUEBLO)

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Table 4-3
Annual Traffic And Revenue Estimates
Scenario 1: I-25N (at Fort Collins) to I-25S (south of Pueblo)
Front Range Toll Road

| Year | Annual <br> Trips (1) | Annual Revenue (1) |  |
| :---: | :---: | :---: | :---: |
|  | (000) |  |  |
| 2010 | 10,001 | \$ | 81,044 |
| 2011 | 10,366 |  | 83,834 |
| 2012 | 10,731 |  | 86,727 |
| 2013 | 11,060 |  | 89,516 |
| 2014 | 11,425 |  | 92,306 |
| 2015 | 11,790 |  | 95,199 |
| 2016 | 12,118 |  | 97,988 |
| 2017 | 12,483 |  | 100,778 |
| 2018 | 12,848 |  | 103,671 |
| 2019 | 13,213 |  | 106,461 |
| 2020 | 13,542 |  | 109,250 |
| 2021 | 13,907 |  | 112,143 |
| 2022 | 14,272 |  | 114,933 |
| 2023 | 14,600 |  | 117,723 |
| 2024 | 14,965 |  | 120,616 |
| 2025 | 15,330 |  | 123,405 |
| 2026 | 15,659 |  | 126,167 |
| 2027 | 15,987 |  | 128,864 |
| 2028 | 16,316 |  | 131,490 |
| 2029 | 16,644 |  | 134,037 |
| 2030 | 16,936 |  | 136,501 |
| 2031 | 17,228 |  | 138,873 |
| 2032 | 17,520 |  | 141,147 |
| 2033 | 17,776 |  | 143,317 |
| 2034 | 18,031 |  | 145,378 |
| 2035 | 18,287 |  | 147,322 |
| 2036 | 18,506 |  | 149,146 |
| 2037 | 18,725 |  | 150,843 |
| 2038 | 18,907 |  | 152,408 |
| 2039 | 19,090 |  | 153,837 |
| 2040 | 19,236 |  | 155,125 |

${ }^{(1)}$ Not adjusted for ramp-up.
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Table 4-4
Annual Traffic And Revenue Estimates

## Scenario 2: I-25N (at Fort Collins) to I-25S (north of Pueblo)

Front Range Toll Road

| Year | Annual <br> Trips (1) | $\begin{gathered} \begin{array}{c} \text { Annual } \\ \text { Revenue (1) } \end{array} \\ \hline(000) \end{gathered}$ |  |
| :---: | :---: | :---: | :---: |
|  | (000) |  |  |
| 2010 | 9,928 | \$ | 76,323 |
| 2011 | 10,257 |  | 79,041 |
| 2012 | 10,622 |  | 81,860 |
| 2013 | 10,987 |  | 84,578 |
| 2014 | 11,315 |  | 87,397 |
| 2015 | 11,680 |  | 90,116 |
| 2016 | 12,045 |  | 92,935 |
| 2017 | 12,374 |  | 95,653 |
| 2018 | 12,739 |  | 98,472 |
| 2019 | 13,067 |  | 101,190 |
| 2020 | 13,432 |  | 104,009 |
| 2021 | 13,797 |  | 106,727 |
| 2022 | 14,126 |  | 109,546 |
| 2023 | 14,491 |  | 112,265 |
| 2024 | 14,856 |  | 115,084 |
| 2025 | 15,184 |  | 117,802 |
| 2026 | 15,513 |  | 120,491 |
| 2027 | 15,841 |  | 123,122 |
| 2028 | 16,170 |  | 125,687 |
| 2029 | 16,498 |  | 128,179 |
| 2030 | 16,827 |  | 130,593 |
| 2031 | 17,119 |  | 132,921 |
| 2032 | 17,411 |  | 135,158 |
| 2033 | 17,703 |  | 137,298 |
| 2034 | 17,958 |  | 139,334 |
| 2035 | 18,214 |  | 141,261 |
| 2036 | 18,433 |  | 143,074 |
| 2037 | 18,652 |  | 144,766 |
| 2038 | 18,871 |  | 146,334 |
| 2039 | 19,053 |  | 147,773 |
| 2040 | 19,236 |  | 149,078 |

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Figure 4-7 shows the two core projects are potential toll facility candidate projects evaluated in the Greater Colorado Springs area. These include Powers Boulevard and the proposed Banning-Lewis Parkway. Several scenarios were evaluated for the different corridors, one of which would combine portions of the two projects.

The Powers Boulevard Corridor would include both potentially completing connections along existing Powers Boulevard to and from I-25 on the north and south and the possibility of upgrading the existing Powers Boulevard from a major arterial to a fully limited access facility.

The proposed Banning-Lewis Parkway would be constructed in a major plan development along the eastern edge of Colorado Springs generally referred to as Banning-Lewis Ranch. That project, if fully built out, would substantially increase the size of the Colorado Springs region. However, most of that planned development is scheduled for subsequent to the year 2020, which results in relatively low early demand for Banning-Lewis Parkway in the early years of the traffic and revenue analysis.

## Proposed Powers Boulevard Corridor

Four scenarios were considered for this corridor. These included:

- Scenario 1 - a new four lane toll road from I-25N (Northgate) to Woodmen Road;
- Scenario 2 - a new four lane toll road from I-25N (Northgate) to Drennan Road;
- Scenario 3 - a new four lane toll road from I-25N (Northgate) to south of Fountaine Boulevard;
- Scenario 4 - a new four lane toll road from I-25N (Northgate) to Woodmen Road plus a new four lane east-west toll road in the Drennan Road corridor connecting I-25 to the Colorado Springs Airport,

The Powers Road project is located in El Paso County and the City Colorado Springs. Powers Boulevard is currently an existing arterial, with at-grade signalized intersections that extend from Research Parkway to Fountain Boulevard, a length of approximately 18.0 miles. A gradeseparated interchange is provided at Platte Avenue (U.S. 24). From Woodmen Road to Platte Avenue, Powers Boulevard is a 6-lane arterial,


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with posted speed limits of 45 MPH ; south of Platte Avenue, Powers Boulevard is reduced to 4-lane section with posted speed limits ranging from 45 to 55 MPH.

The proposed project was studied as a series of phased implementations, represented by four scenarios. In Scenario 1, a new 4-lane toll road would be constructed from I-25 (Northgate) on the north near the Air Force Academy to Woodmen Road; Powers Boulevard, south of Woodmen Road, was assumed to be a freeway. In Scenario 2, the toll road would be extended from Woodmen to Drennan Road, a distance of approximately 11-miles. For Scenario 3, the toll road would be extended south of Drennan Road to a point just south of Fontaine Boulevard, where it would tie into a new east-west arterial providing access to I-25. For all scenarios, it was assumed that a four or six lane frontage road would be provided to replace any existing 'free' capacity, as well as to provide access to local businesses and residences.

Scenario 4, assumed that the north end of the project would be completed as a toll road from Northgate to Woodmen Road. In addition a new eastwest toll road would be built connecting I-25 to the Colorado Springs Airport, running parallel to Drennan Road.

## Current Traffic Conditions

The 2003 ADT on Powers Boulevard just north of Woodmen Road is about 17,000. The volume on Powers, south of Woodmen Road increases to about 35,000 . Woodmen Road itself has an ADT of 27,000 west of Powers and 17,000 on the east side. Woodmen Road also serves as a major east-west facility in this region as indicated by the traffic volumes on Woodmen Road between I-25 and Powers Boulevard, which are in the range of $30,000-35,000$. Moving southward, the ADT on Powers Boulevard changes to about 39,000, just south of Barnes Road. In the central part of Powers Boulevard, traffic volume increases further to over 42,000 south of US-24 (Platte Avenue). Further south, the traffic volumes tend to be relatively lower. On most parts of South Powers Boulevard, daily traffic volumes remain in the range of 10,000 vehicles.

A major north-south facility west of Powers Boulevard is Academy Boulevard, which carries higher traffic volumes. For the most part, the ADT on Academy Boulevard is in the range of 45,000 . It increases in some areas, particularly near Woodmen, to 54,000, and declines to near 30,000 south of Fountain Boulevard. The traffic volumes again increase at its south terminus, near I- 25 to 45,000 vehicles per day.

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Another north-south facility, east of Powers Boulevard, is Marksheffel Road, which passes through rapidly growing areas. However, the current volume on this facility is approximately 7,000 in the northern part, south of Woodmen Road. In the central portion north of the airport, the ADT reduces to 4,000 , which reduces further to 3,000 and less in the southern part of Marksheffel Road east of the Colorado Springs Airport.

## Project Access Points And Tolling Concept

Figure 4-8 presents the assumed project limits and interchange locations for Scenario 1. Major interchanges were assumed at: I-25, Voyager Parkway, Academy Boulevard (S.H.-83), Old Ranch Road, Union Boulevard, Briargate Boulevard, and Research Parkway and Woodman Road.

Scenario 2 extends the toll road further south along Powers Boulevard, to Drennan Road. In addition to the access points in Scenario 1, access to the Project was assumed to be provided at interchanges between Woodmen Road and Drennan Road. These included, Dublin Street, Stetson Hills Boulevard, Barnes Road, Constitution Avenue, Palmer Park Boulevard, Platte Avenue (U.S. 24), Airport Road and Fountain Boulevard, as shown in Figure 4-9.

Scenario 3 is presented in Figure 4-10. Additional access points were assumed to be provided at Grinnel Street, Bradley Road, Fontaine Boulevard and Mesa Ridge Parkway. The tolling concept remains similar to other scenarios.

Scenario 4 combines the north piece described by Scenario 1 and adds a new east-west toll road in the south. The south portion includes an eastwest toll road connecting the Colorado Springs Municipal Airport, to I-25, near Drennan Road. Access point in the south portion is assumed to be provided at South Powers Boulevard. Figure 4-11 presents the assumed access points for both toll roads.

For all four scenarios, electronic tolling was assumed to be implemented by providing toll zones on each mainline segment between each interchange.

## Preliminary Traffic And Revenue Estimates

Toll Rates - Each of the four scenarios was analyzed under varying sets of toll rates for both 2010 and 2030 conditions. The toll rates tested ranged from $\$ 0.05$ to $\$ 0.25$ per mile, at increments of $\$ 0.05$. The results of the analysis produced a toll sensitivity curve for the Project. Based on review of the toll sensitivity curve, the toll rate of $\$ 0.20$ per mile was assumed for





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further analysis including computation of revenue. An estimated proportion of 8 percent was assumed for commercial vehicles. The toll rate for commercial vehicles was considered as three times that of passenger cars.

Estimated Traffic - Figure 4-12 presents future year traffic for Scenario 1. Under 2010 conditions, the expected average daily traffic ranges from 15,000 near I-25 to 27,600 near Woodmen Road. The 2030 counterparts of these volumes are 28,700 and 49,000, respectively.

Figure 4-13 presents 2010 and 2030 traffic estimates for Scenario 2. For opening year, the expected average daily traffic ranges from 13,800 near I25 to 64,500 north of Palmer Park Boulevard. The 2030 counterparts of these volumes are 28,700 and 115,600 , respectively. At the southern terminus, the estimated 2010 and 2030 daily traffic is 43,200 and 66,200, respectively.

Future year traffic estimates for Scenario 3 are shown in Figure 4-14. In 2010, average daily traffic is estimated to range from 13,200 near I-25 to 62,700 north of Palmer Park Boulevard. The 2030 counterparts of these volumes are 27,000 and 113,800, respectively. At the southern terminus, the estimated 2010 and 2030 daily traffic is 14,600 and 26,600 , respectively, south of Fontaine Boulevard.

Figure $4-15$ presents the 2010 and 2030 traffic estimates associated with Scenario 4. In opening year, the expected average daily traffic ranges from 14,100 near I-25 to 26,600, north of Woodmen Road. The 2030 counterparts of these volumes are 28,700 and 49,100 , respectively. In the southern portion, the estimated 2010 and 2030 daily traffic is 17,700 and 31,100 , respectively.

Estimated Annual Trips and Gross Toll Revenue - Table 4-5 presents estimated annual trips and annual gross toll revenues for Scenario 1. Opening year shows an estimated 9.8 million annual trips passing through the Project, generating annual toll revenue of $\$ 11.2$ million. By 2030, the annual trips on the proposed facility increase to 17.6 million, with an annual revenue of $\$ 21.2$ million. The trips and revenue rise to 21.5 and $\$ 26.2$ million in the year 2040

For Scenario 2, an estimated 34.6 million annual trips are anticipated to use the Project in 2010, generating annual toll revenue of $\$ 45.1$ million. By 2030, the number of annual trips on the proposed facility increases to 64.6 million, with an annual revenue of $\$ 82.7$ million. The trips and


Note: All volumes shown are in thousands




Note: All volumes shown are in thousands

ESTIMATED YEAR 2010 AND 2030 AVERAGE DAILY TRAFFIC
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ESTIMATED YEAR 2010 AND 2030 AVERAGE DAILY TRAFFIC



Table 4-5
Annual Traffic And Revenue Estimates
Scenario 1 : I-25N to Woodmen Road
Powers Boulevard Toll Road

| Year | Annual <br> Trips (1) | $\begin{gathered} \begin{array}{c} \text { Annual } \\ \text { Revenue (1) } \end{array} \\ \hline(000) \end{gathered}$ |  |
| :---: | :---: | :---: | :---: |
|  | (000) |  |  |
| 2010 | 9,813 | \$ | 11,231 |
| 2011 | 10,201 |  | 11,731 |
| 2012 | 10,589 |  | 12,230 |
| 2013 | 10,977 |  | 12,729 |
| 2014 | 11,366 |  | 13,229 |
| 2015 | 11,754 |  | 13,728 |
| 2016 | 12,142 |  | 14,227 |
| 2017 | 12,530 |  | 14,727 |
| 2018 | 12,918 |  | 15,226 |
| 2019 | 13,306 |  | 15,725 |
| 2020 | 13,694 |  | 16,225 |
| 2021 | 14,082 |  | 16,724 |
| 2022 | 14,471 |  | 17,223 |
| 2023 | 14,859 |  | 17,723 |
| 2024 | 15,247 |  | 18,222 |
| 2025 | 15,635 |  | 18,721 |
| 2026 | 16,023 |  | 19,220 |
| 2027 | 16,411 |  | 19,720 |
| 2028 | 16,799 |  | 20,219 |
| 2029 | 17,187 |  | 20,718 |
| 2030 | 17,576 |  | 21,218 |
| 2031 | 17,964 |  | 21,717 |
| 2032 | 18,352 |  | 22,216 |
| 2033 | 18,740 |  | 22,716 |
| 2034 | 19,128 |  | 23,215 |
| 2035 | 19,516 |  | 23,714 |
| 2036 | 19,904 |  | 24,214 |
| 2037 | 20,292 |  | 24,713 |
| 2038 | 20,681 |  | 25,212 |
| 2039 | 21,069 |  | 25,712 |
| 2040 | 21,457 |  | 26,211 |

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revenue rise to 79.6 and $\$ 101.5$ million in the year 2040, as summarized in Table 4-6.

Table 4-7 presents the estimated traffic and revenue summary for Scenario 3. This shows that for the opening year, an estimated 42.3 million annual trips would use the Project, generating annual toll revenue of \$52.5 million. By 2030, the number of annual trips on the proposed facility increases to 71.5 million, with an annual revenue of $\$ 96.8$ million. The estimated number of trips and toll revenues are anticipated to rise to 86.1 and $\$ 118.9 .5$ million by 2040.

Table 4-8 presents the estimated traffic and revenue for Scenario 4. For opening year, the estimated number of annual trips using the Project is 14.2 million, producing annual toll revenue of $\$ 15.6$ million. By 2030, the number of annual trips increases to 24.7 million, with an annual revenue of $\$ 29.9$ million. In the last year of analysis, 2040, the number of annual trips is anticipated to rise to 30.0 million with annual toll revenues exceeding \$37.0 million.

## Proposed Banning Lewis Corridor

Two scenarios were considered for the Banning Lewis Corridor. These included:

- Scenario 1 - a new four lane toll road from I-25N (Northgate) to I-25S at Fountaine Boulevard, assuming an unimproved Powers Boulevard; and
- Scenario 2 - a new four lane toll road from I-25N (Northgate) to I-25S at Fountaine Boulevard, assuming an improved Powers Boulevard.

The Banning Lewis project alignment would be located in the eastern portion of Colorado Springs. It would provide a new eastern bypass around the city, as well as providing access to the proposed Banning Lewis Ranch development. The Banning Lewis Ranch Development, if fully realized would cover approximately 21,000 acres, accommodating nearly 75,000 new dwelling units, and nearly 50.0 million square ft of new commercial/office/industrial space.

The Banning Lewis project was studied as new four lane toll road, covering distance of approximately 21.0 miles. At the north end, it was assumed to connect to I-25 at Northgate, and follow the same partial alignment described in Scenario 1 for Powers Boulevard. Just north of Research Parkway, the Banning Lewis alignment would swing east and run parallel to Woodmen Road. The road alignment would then move
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Table 4-6
Annual Traffic And Revenue Estimates
Scenario 2 : I-25N to Drennan Road
Powers Boulevard Toll Road

| Year | Annual <br> Trips (1) | Annual <br> Revenue (1) |  |
| :---: | :---: | :---: | :---: |
|  | (000) |  |  |
| 2010 | 34,582 | \$ | 45,108 |
| 2011 | 36,084 |  | 46,986 |
| 2012 | 37,585 |  | 48,864 |
| 2013 | 39,087 |  | 50,742 |
| 2014 | 40,588 |  | 52,620 |
| 2015 | 42,090 |  | 54,498 |
| 2016 | 43,591 |  | 56,376 |
| 2017 | 45,093 |  | 58,254 |
| 2018 | 46,594 |  | 60,132 |
| 2019 | 48,096 |  | 62,010 |
| 2020 | 49,597 |  | 63,888 |
| 2021 | 51,099 |  | 65,766 |
| 2022 | 52,600 |  | 67,644 |
| 2023 | 54,102 |  | 69,523 |
| 2024 | 55,603 |  | 71,401 |
| 2025 | 57,105 |  | 73,279 |
| 2026 | 58,607 |  | 75,157 |
| 2027 | 60,108 |  | 77,035 |
| 2028 | 61,610 |  | 78,913 |
| 2029 | 63,111 |  | 80,791 |
| 2030 | 64,613 |  | 82,669 |
| 2031 | 66,114 |  | 84,547 |
| 2032 | 67,616 |  | 86,425 |
| 2033 | 69,117 |  | 88,303 |
| 2034 | 70,619 |  | 90,181 |
| 2035 | 72,120 |  | 92,059 |
| 2036 | 73,622 |  | 93,937 |
| 2037 | 75,123 |  | 95,815 |
| 2038 | 76,625 |  | 97,694 |
| 2039 | 78,126 |  | 99,572 |
| 2040 | 79,628 |  | 101,450 |

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Table 4-7
Annual Traffic And Revenue Estimates Scenario 3 : I-25N to Mesa Ridge Parkway Powers Boulevard Toll Road

| Year | Annual <br> Trips (1) | $\begin{gathered} \begin{array}{c} \text { Annual } \\ \text { Revenue (1) } \end{array} \\ \hline(000) \end{gathered}$ |  |
| :---: | :---: | :---: | :---: |
|  | (000) |  |  |
| 2010 | 42,329 | \$ | 52,516 |
| 2011 | 43,788 |  | 54,729 |
| 2012 | 45,247 |  | 56,942 |
| 2013 | 46,705 |  | 59,156 |
| 2014 | 48,164 |  | 61,369 |
| 2015 | 49,623 |  | 63,582 |
| 2016 | 51,081 |  | 65,796 |
| 2017 | 52,540 |  | 68,009 |
| 2018 | 53,999 |  | 70,222 |
| 2019 | 55,457 |  | 72,436 |
| 2020 | 56,916 |  | 74,649 |
| 2021 | 58,375 |  | 76,862 |
| 2022 | 59,833 |  | 79,076 |
| 2023 | 61,292 |  | 81,289 |
| 2024 | 62,751 |  | 83,502 |
| 2025 | 64,209 |  | 85,716 |
| 2026 | 65,668 |  | 87,929 |
| 2027 | 67,127 |  | 90,142 |
| 2028 | 68,585 |  | 92,356 |
| 2029 | 70,044 |  | 94,569 |
| 2030 | 71,503 |  | 96,782 |
| 2031 | 72,961 |  | 98,996 |
| 2032 | 74,420 |  | 101,209 |
| 2033 | 75,879 |  | 103,422 |
| 2034 | 77,338 |  | 105,636 |
| 2035 | 78,796 |  | 107,849 |
| 2036 | 80,255 |  | 110,062 |
| 2037 | 81,714 |  | 112,275 |
| 2038 | 83,172 |  | 114,489 |
| 2039 | 84,631 |  | 116,702 |
| 2040 | 86,090 |  | 118,915 |

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Table 4-8
Annual Traffic And Revenue Estimates Scenario 4 : I-25N to Woodmen \& Airport to I-25S

Powers Boulevard Toll Road

| Year | Annual <br> Trips (1) | $\begin{gathered} \text { Annual } \\ \text { Revenue (1) } \\ \hline(000) \end{gathered}$ |  |
| :---: | :---: | :---: | :---: |
|  | (000) |  |  |
| 2010 | 14,165 | \$ | 15,605 |
| 2011 | 14,693 |  | 16,320 |
| 2012 | 15,221 |  | 17,036 |
| 2013 | 15,750 |  | 17,751 |
| 2014 | 16,278 |  | 18,466 |
| 2015 | 16,807 |  | 19,182 |
| 2016 | 17,335 |  | 19,897 |
| 2017 | 17,864 |  | 20,613 |
| 2018 | 18,392 |  | 21,328 |
| 2019 | 18,920 |  | 22,043 |
| 2020 | 19,449 |  | 22,759 |
| 2021 | 19,977 |  | 23,474 |
| 2022 | 20,506 |  | 24,189 |
| 2023 | 21,034 |  | 24,905 |
| 2024 | 21,563 |  | 25,620 |
| 2025 | 22,091 |  | 26,335 |
| 2026 | 22,619 |  | 27,051 |
| 2027 | 23,148 |  | 27,766 |
| 2028 | 23,676 |  | 28,482 |
| 2029 | 24,205 |  | 29,197 |
| 2030 | 24,733 |  | 29,912 |
| 2031 | 25,262 |  | 30,628 |
| 2032 | 25,790 |  | 31,343 |
| 2033 | 26,318 |  | 32,058 |
| 2034 | 26,847 |  | 32,774 |
| 2035 | 27,375 |  | 33,489 |
| 2036 | 27,904 |  | 34,204 |
| 2037 | 28,432 |  | 34,920 |
| 2038 | 28,961 |  | 35,635 |
| 2039 | 29,489 |  | 36,351 |
| 2040 | 30,018 |  | 37,066 |

[^13]Wilbur Smith Associates
south running parallel to Marksheffel Road, extending to Drennan Road. From Drennan Road, the road alignment would move in a western direction, terminating at I-25. Two scenarios were studied based on this alignment. Scenario 1 assumed that Powers Boulevard would not be improved; while Scenario 2 assumed that Powers Boulevard would be upgraded to a freeway.

## Current Traffic Conditions

The primary north-south facility is Marksheffel Road, which carries traffic volumes ranging from 7,000 in its northern sections, to approximately 4,000 in its central portion, near the Colorado Springs airport. The daily traffic volumes drop to 3,000 south of the airport.

The primary east-west route is Woodmen Road, north of the study area, which presently services approximately 17,000 vpd. Other east-west routes include Barnes Road, which carries traffic in range of $16,000 \mathrm{vpd}$; and Constitution Avenue which carries traffic in the range of 10,000 to 30,000 vpd.

Areas east of Marksheffel Road, which constitute the Banning-Lewis corridor, are not currently developed to the extent that they would result in major traffic impacts on Marksheffel Road at this time.

## Project Access Points And Tolling Concept

Figure 4-16, presents the assumed interchange access location points for the Project. Access to the Project is assumed to be provided from all major interchanges between I-25N, and the southern terminus of the Project where it connects back to I-25S. Starting from the north terminus, the interchanges assumed include, S.H. 83, Old Ranch Road, Union \& Briargate Boulevard, Vollmer Road, East Woodmen Road, U.S.24, S.H. 94, Marksheffel Road and South Powers Boulevard. The electronic toll collection scheme assumed comprises a toll zone between each interchange as illustrated in the above referenced figure.

## Preliminary Traffic And Revenue Estimates

Toll Rates - The toll rates tested ranged from $\$ 0.05$ to $\$ 0.25$ per mile, at increments of $\$ 0.05$. For every toll rate tested a unique revenue yield was produced, from which a toll sensitivity curve was developed. Based on review of toll sensitivity curves for 2010 and 2030, a toll rate of $\$ 0.20$ per mile was assumed for further analysis including computation of revenue.

Estimated Traffic - Figure 4-17 presents the estimated 2010 and 2030 traffic for Scenario 1. In the opening year, the expected average daily traffic ranges from 7,500 near I-25 to 3,800 near East Woodmen Road.



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The 2030 counterparts of these volumes are 22,500 and 13,200 , respectively. Higher volume is expected south of S.H. 94, which is 6,800 in 2010 and 19,900 in 2030. At the south terminus, near I-25S, the volumes are 7,500 and 13,500 for 2010 and 2030, respectively.

Figure 4-18 presents the future year traffic Scenario 2. This shows that in 2010, the expected average daily traffic ranges from 15,100 near I-25 to 2,700 near East Woodmen Road. The 2030 counterparts of these volumes are 29,800 and 8,600 , respectively. Higher volume is expected south of S.H.-94, which is 4,500 in 2010 and 12,500 in 2030. At the south terminus, near I-25S, the volumes are 9,800 and 15,800 for 2010 and 2030, respectively.

It is important to recognize that the relatively low traffic volumes estimated for the Banning-Lewis toll road reflect the fact that the models used in the analysis assumed no more than 30.0 percent of the ultimate planned development for Banning-Lewis Ranch would be in place by 2030. This was the most distant modeling year available for use in the analysis. In addition, the level of model detail in the Banning-Lewis Ranch area was relatively low, hence this may have resulted in a slight underestimation of local trip demand within the Banning-Lewis Ranch, coupled with the assumed early stages of development, resulted in relatively low traffic and revenue potential on that facility.

In this particular case, the longer term traffic and revenue potential for the project may well be much higher, depending on the pace and ultimate level of development in the Banning-Lewis Ranch project. This should be carefully monitored; and study findings updated as more specific information about the planned development, including anticipated timing, becomes known.

Estimated Annual Trips and Gross Toll Revenue - Table 4-9 presents the traffic and revenue summary for Scenario 1. For opening year, the number of annual trips using the Banning Lewis project is estimated to be 10.1 million, producing annual toll revenue of $\$ 10.5$ million. By 2030, the annual trips on the proposed facility increases to 23.9 million, generating annual revenue of $\$ 28.7$ million. In 2040, the number of annual trips is expected to reach 30.9 million, generating annual toll revenue of $\$ 37.8$ million.

Future year traffic and revenues for Scenario 2 are shown in Table 4-10. In the opening year, the annual number of trips is expected to be 13.7 million, producing approximately 12.8 million in revenue. By 2030, the number of annual trips is estimated to increase to 26.2 million, while toll

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Table 4-9
Annual Traffic And Revenue Estimates Scenario 1 : Unimproved Powers Blvd. Banning-Lewis Parkway

| Year | Annual <br> Trips (1) | Annual Revenue (1) |  |
| :---: | :---: | :---: | :---: |
|  | (000) | (000) |  |
| 2010 | 10,095 | \$ | 10,486 |
| 2011 | 10,788 |  | 11,395 |
| 2012 | 11,481 |  | 12,305 |
| 2013 | 12,174 |  | 13,214 |
| 2014 | 12,867 |  | 14,124 |
| 2015 | 13,560 |  | 15,034 |
| 2016 | 14,253 |  | 15,943 |
| 2017 | 14,946 |  | 16,853 |
| 2018 | 15,639 |  | 17,762 |
| 2019 | 16,332 |  | 18,672 |
| 2020 | 17,025 |  | 19,582 |
| 2021 | 17,718 |  | 20,491 |
| 2022 | 18,411 |  | 21,401 |
| 2023 | 19,104 |  | 22,310 |
| 2024 | 19,797 |  | 23,220 |
| 2025 | 20,490 |  | 24,130 |
| 2026 | 21,183 |  | 25,039 |
| 2027 | 21,876 |  | 25,949 |
| 2028 | 22,569 |  | 26,858 |
| 2029 | 23,262 |  | 27,768 |
| 2030 | 23,955 |  | 28,678 |
| 2031 | 24,648 |  | 29,587 |
| 2032 | 25,341 |  | 30,497 |
| 2033 | 26,034 |  | 31,406 |
| 2034 | 26,727 |  | 32,316 |
| 2035 | 27,420 |  | 33,226 |
| 2036 | 28,113 |  | 34,135 |
| 2037 | 28,806 |  | 35,045 |
| 2038 | 29,499 |  | 35,954 |
| 2039 | 30,192 |  | 36,864 |
| 2040 | 30,885 |  | 37,774 |

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Table 4-10
Annual Traffic And Revenue Estimates
Scenario 2 : Powers Blvd. as Freeway Banning-Lewis Parkway

| Year | Annual <br> Trips (1) | Annual <br> Revenue (1) <br> $(000)$ |  |
| :---: | :---: | :---: | :---: |
|  | (000) |  |  |
| 2010 | 13,698 | \$ | 12,790 |
| 2011 | 14,325 |  | 13,521 |
| 2012 | 14,953 |  | 14,251 |
| 2013 | 15,581 |  | 14,982 |
| 2014 | 16,208 |  | 15,712 |
| 2015 | 16,836 |  | 16,443 |
| 2016 | 17,464 |  | 17,173 |
| 2017 | 18,091 |  | 17,904 |
| 2018 | 18,719 |  | 18,634 |
| 2019 | 19,346 |  | 19,365 |
| 2020 | 19,974 |  | 20,095 |
| 2021 | 20,602 |  | 20,826 |
| 2022 | 21,229 |  | 21,556 |
| 2023 | 21,857 |  | 22,287 |
| 2024 | 22,485 |  | 23,017 |
| 2025 | 23,112 |  | 23,748 |
| 2026 | 23,740 |  | 24,478 |
| 2027 | 24,368 |  | 25,209 |
| 2028 | 24,995 |  | 25,939 |
| 2029 | 25,623 |  | 26,670 |
| 2030 | 26,251 |  | 27,400 |
| 2031 | 26,878 |  | 28,131 |
| 2032 | 27,506 |  | 28,861 |
| 2033 | 28,133 |  | 29,592 |
| 2034 | 28,761 |  | 30,322 |
| 2035 | 29,389 |  | 31,053 |
| 2036 | 30,016 |  | 31,783 |
| 2037 | 30,644 |  | 32,514 |
| 2038 | 31,272 |  | 33,244 |
| 2039 | 31,899 |  | 33,975 |
| 2040 | 32,527 |  | 34,705 |

[^15]Wilbur Smith Associates
revenues are estimated to rise to $\$ 27.4$ million. In the final year of analysis, the Project is estimated to service 32.5 million trips annually, and generate $\$ 34.7$ million in annual toll revenue.

## PROPOSED I-70 MOUNTAIN CORRIDOR

A total of five scenarios were studied for the I-70 Mountain Corridor. These included:

- Scenario 1 - Two lane reversible express toll project from west of the Eisenhower Tunnel to Floyd Hill. Add new bores at the Eisenhower and Twin Tunnels;
- Scenario 2 - Add one general purpose lane in both directions from Eisenhower Tunnel to Floyd Hill. Add new bores at Eisenhower and Twin Tunnels. Collect tolls in a one-way direction. $\$ 5.00$ toll pays for cost of tunnels and roadways;
- Scenario 3 - Add one general purpose lane in both directions from Eisenhower Tunnel to Floyd Hill. Add new bores at Eisenhower and Twin Tunnels. Collect tolls in a one-way direction. $\$ 5.00$ toll pays for cost of tunnels only;
- Scenario 3a - Add one general purpose lane in both directions from Eisenhower Tunnel to Floyd Hill. Add new bores at Eisenhower and Twin Tunnels. Collect tolls in a one-way direction. $\$ 3.00$ toll pays for cost of tunnels only; and
- Scenario 3b - Add one general purpose lane in both directions from Eisenhower Tunnel to Floyd Hill. Add new bores at Eisenhower and Twin Tunnels. Collect tolls in a one-way direction. $\$ 2.00$ toll pays for cost of tunnels only;

These projects covered improvements to I-70 generally between the Eisenhower Tunnel and Floyd Hill, representing a length of approximately $35-$ miles, as shown in Figure $4-19$. It should be noted that these five scenarios were developed for analysis purposes only. The WSA study team recognizes that Colorado law precludes tolling of existing capacity, but Federal law allows tolling of existing bridges and tunnels for reconstruction or for providing additional capacity. Within this section of I-70, the current section of roadway is generally two lanes per direction with steep uphill and downhill grades. In addition, there are two tunnels:


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the Eisenhower Tunnel, located just west of the Loveland Bypass (U.S. 6); and the Twin Tunnels near Idaho Springs.

## Current Traffic Conditions

I-70, between the Eisenhower Tunnel and Floyd Hill presently services AADT volumes in the range of 30,000 to 40,000 . The highest traffic volumes are seen at Twin Tunnels location, between S.H. 70 Idaho Springs (Interchange 243) and Hidden Valley (Interchange 244). The peak monthly travel demand on this section of I-70 occur during the summer months of July and August, which may be 25.0 percent higher than average month in the year. The shoulder months of June and September are also higher than average. The winter months, January-March, are also relatively high due to the ski season recreational trends.

In the summer months, weekend travel is typically heavier than weekday travel. In summer, for example, Sunday traffic through the Twin Tunnels can reach almost $65,000 \mathrm{vpd}$, as compared to the annual average of 40,000. Saturday peaks are almost as high. The typical weekday traffic outside of the summer months is well below average daily traffic.

The highest hourly traffic volumes are typically experienced on Saturdays and Sundays, particularly during the midday time periods in the westbound and eastbound travel directions, respectively.

## Project Access Points and Tolling Concepts

For Scenario 1, the reversible express toll lanes, access was assumed to be provided at five points from the general purpose lanes. Traffic traveling westbound, motorists would initially be able to access the express toll lanes just east of the U.S. 6 Clear Creek Interchange. The second access point was assumed to be located west of the U.S. Clear Creek Interchange. The third fully directional access point was assumed to be provided east of the U.S. 40 Empire Interchange, approximately 10.0 miles west from the last access point. The next access point was assumed to be located approximately 15.0 miles west, between the Bakersville and U.S. 6 interchanges. The last access/egress point to the express toll lanes was assumed to be situated just west of the Eisenhower Tunnel. An ETC toll zone would be located in the express toll lanes for each of the mainline segments between the access/egress points. Figure $4-20$ presents the access/egress points to the express toll lanes and the location for each of the toll zones.

For Scenarios 2, 3, 3a and 3b all current interchanges would be maintained. Tolling was assumed to occur in one direction only at each of the tunnels. The tolls were assumed to be collected in opposite directions.



For conceptual purposes, the toll plazas at the Eisenhower Tunnel and Twin Tunnels were shown in the eastbound direction and westbound directions, respectively. More detailed studies would be conducted to establish the most feasible location. Figure 4-21 presents the concept.

## Preliminary Traffic and Revenue Estimates

Toll Rates - For Scenario 1, the reversible express toll lanes, a series of toll rates were tested for weekday, Saturday and Sunday by time period. Tolls were tested ranging from $\$ 0.05$ to $\$ 0.50$ per-mile. For each discrete time period, toll sensitivity curves were prepared, and an optimum toll rate selected.

Table 4-11 presents the toll rates for Scenario 1, for each of the time periods by day and year. For 2010, weekday rates were no higher than $\$ 0.05$ per-mile. On Saturdays, the same year, the highest toll rate would be in the westbound midday period at $\$ 0.15$ per-mile; while on Sunday optimum toll rates of $\$ 0.10$ and $\$ 0.15$ per-mile would be charged in the eastbound direction for the midday and evening time periods. In 2025, toll rates would remain unchanged from 2010, for the weekday period, reflecting minimum demand. However both Saturday and Sunday would see toll rates as high as $\$ 0.50$ per-mile in the busiest time periods.

For Scenarios 2, 3, 3a and 3b a flat toll charge was assumed to be levied at the Eisenhower and Twin Tunnels in a one-way direction to all traffic. The toll rates tested included a $\$ 2.00$, $\$ 3.00$ and $\$ 5.00$ passenger car rate.

Estimated Traffic - Year 2010 traffic conditions for Scenario 1 are presented in Figures 4-22, 4-23 and 4-24 for weekday, Saturday and Sunday, respectively. Review of the weekday data shows minimal demand in the express toll lanes. The analysis for Saturday shows higher demand in the express toll lanes in the westbound direction for AM and Midday time periods. For Sunday, similar traffic volumes are seen in the eastbound direction occurring in the Midday and PM time periods.

Year 2025 traffic conditions for Scenario 1 are presented in Figures 4-25, 4-26 and 4-27 for weekday, Saturday and Sunday, respectively. Minimal demand in the express lane would be anticipated during the weekdays. For Saturday, the higher demand would occur in the AM and Midday time periods in the westbound direction. The analysis shows that the highest travel demand is likely to occur during Sunday for the Midday and PM time periods. While travel demand is likely to be high for both weekend days for specific periods, the relatively high toll rates would temper demand within the express toll lanes and allow them to operate in a free




Legend
General Purpose LanesTunnels
Mainline Toll Plazas. One Direction Only.

Toll Locations I-70 Mountain Corridor
Scenarios 2,3,3-A,3-B : One Way Tolling at Eisenhower, Idaho Springs Tunnels
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| LEGEND |  |  |
| :--- | :---: | :--- |
| 0.0 | Existing General | AMPK | A.M. Peak (6-10 A.M.)

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| LEGEND |  |  |
| :--- | :---: | :--- |
| 0.0 | Existing General | AMPK | A.M. Peak (6-10 A.M.) 1 (10 A.M. - 3 P.M.)



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flow manner at the expense of the general purpose lanes, which would be severely degraded.

The future year traffic estimates for Scenarios 2, 3, 3a, and 3b are shown in Figure 4-28 for 2010. At 2010, daily traffic estimates at the Twin Tunnels for the tolled direction are $27,800,30,300$ and 31,500 for the $\$ 5.00, \$ 3.00$ and $\$ 2.00$ passenger car toll, respectively. Similarly at the Eisenhower Tunnel, traffic estimates in the tolled direction are 16,400 , 19,400 and 21,200 for the same three toll rates.

Figure 4-29 presents 2025 average daily traffic estimates for Scenarios 2, 3, 3a, and 3b. At the Twin Tunnels, daily traffic estimates for the tolled direction are $32,600,36,000$ and 37,400 , which correspond to the $\$ 5.00$, $\$ 3.00$ and $\$ 2.00$ passenger car toll rates, respectively. At the Eisenhower Tunnel, the traffic estimates for the same three toll rates are 19,100, 23,000 and 24,800 vpd, respectively.

Estimated Annual Trips/Transactions and Gross Toll Revenue - Table 412 presents the annual trips and toll revenues for Scenario 1 from 2010 to 2040. In the opening year, the number of annual trips using the Project is estimated to be 1.9 million, generating approximately $\$ 5.8$ million in toll revenue. By 2025, the number of annual trip increases to 2.8 million, producing approximately $\$ 29.5$ million. The final year, 2040, shows the number of annual trips using the Project exceeding 8.2 million with corresponding toll revenue of approximately $\$ 87.1$ million.

Table 4-13 presents the annual toll transactions and annual gross toll revenues for Scenario's 2, 3, 3a and 3b. At the $\$ 5.00$ passenger car toll rate, opening year revenues are estimated to reach $\$ 95.3$ million. By 2025, annual toll revenues increase to $\$ 111.7$ million. For the last year shown, 2040, annual revenues are anticipated to reach $\$ 130.8$ million.

Opening year, annual gross toll revenue for Scenario 3-A, is estimated to be $\$ 64.4$ million. By 2025, this is estimated to increase to $\$ 76.3$ million. Year 2040 shows annual toll revenues reaching $\$ 90.3$ million.

For Scenario 3b, the $\$ 2.00$ passenger car toll, opening year toll revenue is estimate to be $\$ 45.6$ million. In 2025, the tunnels are estimated to generate $\$ 53.7$ million in toll revenue. In the last year shown, annual gross toll revenue is anticipated to reach $\$ 63.3$ million.



## Legend

General Purpose LanesTunnels
Mainline Toll Plazas. One Direction Only.

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Table 4-12
Annual Traffic And Revenue Estimates
Scenario 1: Reversible ETL, from west of Eisenhower
Tunnel to Floyd Hill
I-70 Mountain Corridor

| Year | Annual <br> Trips (1) | Annual <br> Revenue (1) |  |
| :---: | :---: | :---: | :---: |
|  |  |  | 00) |
| 2010 | 1,871 | \$ | 5,752 |
| 2011 | 1,920 |  | 6,414 |
| 2012 | 1,972 |  | 7,152 |
| 2013 | 2,024 |  | 7,975 |
| 2014 | 2,079 |  | 8,893 |
| 2015 | 2,134 |  | 9,917 |
| 2016 | 2,191 |  | 11,058 |
| 2017 | 2,250 |  | 12,331 |
| 2018 | 2,310 |  | 13,750 |
| 2019 | 2,371 |  | 15,332 |
| 2020 | 2,435 |  | 17,097 |
| 2021 | 2,500 |  | 19,065 |
| 2022 | 2,567 |  | 21,259 |
| 2023 | 2,635 |  | 23,706 |
| 2024 | 2,706 |  | 26,435 |
| 2025 | 2,778 |  | 29,477 |
| 2026 | 3,084 |  | 32,722 |
| 2027 | 3,408 |  | 36,162 |
| 2028 | 3,749 |  | 39,781 |
| 2029 | 4,105 |  | 43,564 |
| 2030 | 4,475 |  | 47,489 |
| 2031 | 4,856 |  | 51,530 |
| 2032 | 5,245 |  | 55,658 |
| 2033 | 5,639 |  | 59,837 |
| 2034 | 6,034 |  | 64,032 |
| 2035 | 6,427 |  | 68,200 |
| 2036 | 6,813 |  | 72,299 |
| 2037 | 7,189 |  | 76,282 |
| 2038 | 7,549 |  | 80,103 |
| 2039 | 7,889 |  | 83,716 |
| 2040 | 8,205 |  | 87,072 |

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## Chapter 5

## Preliminary Project Cost Estimates

## GENERAL METHODOLOGY

This section describes the approach used to estimate roadway capital costs and annual operations and maintenance ( $\mathrm{O} \& \mathrm{M}$ ) costs for the candidate toll projects for the second-tier study. In the first-tier study, the necessary roadway improvements were identified at a sketch-planning level to meet each individual corridor development plan. Program-planning level cost estimates were developed by using unit cost per lane-mile factors and all costs were expressed as ranges. For each improvement type, the unit-costs per lane-mile were developed to represent typical applications and were adjusted appropriately for special considerations such as major bridge crossings and interchanges. The construction costs for each project were then compared with a relative measure of the project's projected toll revenue to determine its Relative Feasibility Index.

Within the second-tier study, more detailed cost estimates were developed for a smaller, refined list of selected projects found to warrant further study in the first-tier study, using recent bid tabulations and other construction cost-related data to create unit cost build up tables based on similar CDOT roadway projects. The second-tier study was still considered a preliminary feasibility analysis. The analyses were not conducted to a sufficient level of detail to be used in support of actual project financing, but were of sufficient precision to identify those projects or elements of project corridors that were potentially feasible as toll facilities and could warrant further study at an investment grade study level as part of the project implementation process. All cost analyses were estimated in current 2004 dollars and cost inflationary factors and the additional costs associated with toll collection facilities were applied if cost estimates from previous studies or reports were used.

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As part of the second-tier study, the toll collection system capital, operations and maintenance costs were estimated. Although the toll collection system capital costs are always a small percentage of the toll facility construction (i.e., capital) costs, the toll system always provides some schedule completion risk, potentially delaying the start of revenue operations. A significant component of this risk is the complexity of the System. Since there is considerable variation on toll systems capital costs, component identification and unit pricing accomplishes both a more complete understanding of the system design and a price that is within a reasonable realm of possibilities, given a number of unknowns.

System capital costs are subdivided into multiple distinct categories, each with multiple unit items deemed to have a high probability of being implemented. Item quantities are derived from the number of tolling points, length of the facility, and location of the facility. The same process was used for developing operations and maintenance costs, but with only two categories. However, operations costs are dominated by the electronic toll collection (ETC) costs derived from modeled traffic and trip data and converted to an annual cost using an industry supported per trip unit price. Conversely, violation transactions, the single alternative to ETC trip transactions, are assumed to derive revenue from issued citations that exactly equals all costs incurred to processing the violation. Except for the first year of operations, this has proved to be a valid assumption since the Agency can adjust operations as needed. All cost analyses were estimated in current 2004 dollars.

## ESTIMATED ROADWAY CAPITAL COSTS

For the roadway capital costs, a review of existing and planned roadway infrastructure was performed to determine the extent and nature of the existing roadway infrastructure. The necessary roadway improvements were then determined to meet each corridor's proposed development plan. These typical roadway characteristics were developed based on current CDOT standards and AASHTO guidelines. For those projects where an environmental study has been recently completed or is currently ongoing, adjustments were made to these characteristics/parameters based on the assumptions made in the corridor/EIS studies or recently completed construction. The Colorado Department of Transportation's geographic information systems (GIS) database was used to characterize the existing conditions of each candidate toll project, as well as windshield-surveys.

Utilizing available project cost information from Colorado for similar facilities, cost estimates from earlier studies, and previous cost estimation

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experience, unit cost factors were developed for each improvement type to represent the corridor improvement costs. Capital cost estimates included grading, drainage, surfacing and paving for an interstate-type facility. In addition, unit costs were developed for interchanges, bridges and other structures such as elevated ramps and retaining walls. Terrain conditions were identified based on available information from the Colorado Department of Transportation GIS database. Other incidental costs included consideration of erosion control, signing and pavement marking, maintenance of traffic during construction, traffic control and mobilization, construction staking and inspection and utility relocations.

Appropriate add-ons for "soft" costs associated with engineering design, right-of-way acquisition, and program management and administration were considered to develop a total capital cost. A contingency of 20 percent was added to each project to account for design unknowns. All cost analyses were estimated in current 2004 dollars and cost inflationary factors and the additional costs associated with toll collection facilities were applied if cost estimates from previous studies or reports were used in the study. A table showing the assumption and description for each unit cost factor is shown in Appendix A.

For each project, the type and location of access points for the toll system was taken into account as a part of the capital cost estimates. The beginning and ending of each toll system was assumed to have a transition area between the general purpose lanes and the express toll lanes, in order to provide time and distance to add or drop the express toll lanes. Figure $5-1$ shows an example application for a transition area.

The majority of the access to the express toll lanes was assumed to occur through the use of slip toll access points located between existing interchanges. Figure 5-2 shows a typical application of slip toll access for barrier-separated express toll lanes located in the median of an existing roadway. For direct system connections, such as between the toll system of I-70 East and I-225, direct ramp toll access through flyover ramps was assumed. Figure 5-3 shows an example of direct ramp toll access. For each project, the location of transition areas, slip and direct toll access can be seen on each project's individual information sheet, shown in several Figures throughout the remainder of this chapter. For those projects on new alignment, such as the Front Range project, all capacity would be tolled through electronic toll collection so no exclusive toll access is required.

To provide flexibility in the evaluation of a corridor's financial feasibility, a range of construction improvements or "scenarios" were provided as




necessary on a corridor-by-corridor basis. This provision allowed for adjustments to the facility type, improvements or limits to maximize the potential financial viability of a corridor or corridors, depending on corridor packaging. By providing a range of scenarios for a particular corridor, appropriate considerations can then be given to the sensitivity of a corridor's financial feasibility to the cost side of the feasibility equation. The following section describes the cost methodology and assumptions for each candidate toll project.

## I-25 North Scenario 1

As shown in Figure 5-4, the I-25 North Scenario 1 project spans approximately 26 miles between S.H. 66 and U.S. 36. The project is subdivided into two sections with different improvement types. From S.H. 66 to $120^{\text {th }}$ Avenue, I- 25 would have three general purpose lanes and two express toll lanes in each direction. From $120^{\text {th }}$ to U.S. 36, I-25 would have three general purpose lanes in each direction and two reversible express toll lanes. A separate ongoing study is looking at the feasibility of converting the existing two-lane reversible high-occupancy vehicle (HOV) facility from US 36 to downtown Denver to a two-lane reversible highoccupancy toll (HOT) facility. The WSA study team also recognizes that segments of I-25 have recently been widened, resulting in the tolling of existing capacity. However, the project configuration assumptions were prepared for analysis purposes only.

In both sections the express toll lanes would be located in the median of the existing roadway. From $120^{\text {th }}$ Avenue to US 36, the reversible express toll lanes would be separated from the general purpose lanes by a concrete barrier. However, from SH 66 to $120^{\text {th }}$ Avenue, the express toll lanes would be separated by a four-foot buffer in order to fit the proposed section within the existing median width. In addition, the buffer allows flexibility for traffic to merge back into the general purpose lanes in the case of an incident.

For the section of I-25 from US 36 to S.H.7, it was assumed that the existing general purpose lanes would need to be reconstructed because the current median width is insufficient to add express toll lanes in the median without impacting the general purpose lanes. The section of I-25 from S.H. 7 to SH 66 has sufficient median width that will allow express toll lanes to be added in the median without significant impacts to the general purpose lanes. The existing right-of-way along the majority of the corridor was within a range of 200 to 300 feet. In order to construct the express toll lanes in the section from US 36 to $120^{\text {th }}$ Avenue, it was assumed that right-of-way would need to be purchased at several locations along the corridor. The proposed section shown in Figure 5-4 provides the widths

## Project Length: 26 Miles

Project Type: Express Toll Lanes/Reversible Express Toll Lanes/HOT Lanes
Description: Tolling new capacity and Conversion of HOV to HOT
Improvement Scenario: Add 2 express toll lanes each direction
from SH 66 to just $N$ of 120th; add 2 reversible
from SH 66 to just $N$ of 120 th; add 2 reversible convert existing HOV to HOT from 84th to US 36

Existing Corridor Attributes: Number of Lanes:

- 2 each direction (SH 66 to SH 7 )

3 each direction (SH 7 to US 36)
Outside Shoulder Width: Varies $8^{\prime}$ to $10^{\prime}$
Inside Shoulder Width: Varies 2' to 10'
Median Width:
$-52^{\prime}$ (SH 66 to SH 7 )
4' (SH 7 to US 36)
Terrain: Rolling


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for lanes, median treatments and shoulders assumed for the improved roadway.

No new interchanges were assumed for the corridor, but most existing interchanges required ramp or bridge reconstruction in order to construct the express toll lanes. As shown on Figure 5-4, the majority of the express toll lane access along the corridor would be provided through slip toll access between existing interchanges. The proposed reversible HOT facility was assumed to connect to the existing I- 25 HOV facility at US 36. In addition, the beginning and ending sections of the express toll facility at S.H. 66 would have direct access and a transition area similar to the example shown in Figure 5-1. This would be true for the beginning and ending sections of all the projects which add express toll lanes in the median of an existing roadway.

## I-25 North Scenario 2

The I-25 North Scenario 2 project limits extend from S.H. 7 to U.S. 36 for a distance of approximately 12 miles. From S.H. 7 to U.S. 36, I-25 would have three general purpose lanes in each direction and two reversible express toll lanes. As mention above, a separate ongoing study is looking at the feasibility of converting the existing two-lane reversible highoccupancy vehicle (HOV) facility from US 36 to downtown Denver to a two-lane reversible high-occupancy toll (HOT) facility.

Similar to Scenario 1 the reversible express toll lanes would be located in the median of the existing roadway and would be separated from the general purpose lanes by concrete barrier. It was assumed that the existing general purpose lanes would need to be reconstructed because the current median width is insufficient to add express toll lanes in the median without impacting the general purpose lanes. The existing right-of-way along the majority of the corridor was within a range of 200 to 300 feet. In order to construct the express toll lanes, it was assumed that right-of-way would need to be purchased at several locations along the corridor.

The proposed section shown in Figure 5-5 provides the widths for lanes, median treatments and shoulders assumed for the improved roadway. No new interchanges were assumed for the corridor, but some existing interchanges required ramp or bridge modifications in order to construct the express toll lanes. As shown on Figure 5-5, the majority of the express toll lane access along the corridor would be provided through slip toll access between existing interchanges. The proposed reversible HOT facility was assumed to connect to the existing I-25 HOV facility at U.S. 36.

Project Limits: US 36 (begin $\log 217$ ) to SH 7 (end $\log 229$ )
Project Length: 12 Miles
Project Type: Reversible Express Toll Lanes/HOT Lanes
Description: Tolling new capacity and Conversion of HOV to HOT
Improvement Scenario: Add 2 reversible express toll lanes from SH 7 to 84 th; convert existing HOV to HOT
from 84 th to US 36 .

Existing Corridor Attributes: Number of Lanes:
-3 each direction (SH 7 to US 36) Outside Shoulder Width: Varies $8^{\prime}$ to 10 nside Shoulder Width: Varies $4^{\prime}$ to $1{ }^{\prime}$
edian Width:
(SH 7 to US 36 )
Terrain: Rolling
Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling


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## Wilbur Smith Associates <br> I-70 East Scenario 1

The I-70 East Scenario 1 project is located between I-25 and E-470. The project is approximately 12 miles and is subdivided into two sections with different improvement types. From I-25 to just east of I-270, the section would have three general purpose lanes in each direction, the majority of which is on elevated structure, and two express toll lanes each direction on elevated structure, located adjacent to the existing I-70 alignment on the north side. From just east of I-270 to just west of E-470, I-70 would vary between two (east of Chambers to E-470) and three (east of I-270 to east of Chambers) general purpose lanes and two express toll lanes in each direction located at-grade. Within this section the express toll lanes would be located in the median of existing I-70 and would be separated from the general purpose lanes by a concrete barrier. It was assumed that the existing general purpose lanes would need to be reconstructed between I270 and E-470 because the current median width is not sufficient to add express toll lanes in the median without impacting the general purpose lanes.

The existing right-of-way along the eastern portion of the project from I270 to E-470 was assumed to be adequate and no new right-of-way was assumed to be purchased. However, right-of-way from I-25 to I-270 was assumed to be acquired since the section was located on elevated structure to the north of the existing I-70 alignment. The proposed section shown in Figure 5-6 provides the widths for lanes, median treatments and shoulders assumed for the improved roadway.

For the elevated roadway structure, access points were assumed to be needed at Colorado Boulevard, Quebec and I-270 through direct drop down ramps. For the section at-grade, no new interchanges were assumed for the corridor, but some existing interchanges required ramp or bridge modifications in order to construct the express toll lanes. Most of the mainline bridges in this section were assumed to be widened to accommodate the express toll lanes. As shown on Figure 5-6, access along the eastern portion of the corridor would be provided through slip toll access between existing interchanges. Direct connections via dedicated flyover ramps were assumed at I-225 and I-270 to and from the express toll lanes on I-70 to the general purpose lanes on I-225 and I-270.

Information from the ongoing I-70 Environmental Impact Statement was used to develop the cost estimate for the project.

## I-70 East Scenario 2

The I-70 East Scenario 2 project is located between I-25 and Chambers. The project is approximately 8 miles and is subdivided into two sections

Project Limits: 1 -25 (begin Log 274) to E-470 (end Log 289)
Project Length: 13 Miles
Project Type: Express Toll Lanes
Description: Tolling new capacity

Improvement Scenario: Add 2 express toll lanes each direction.
Existing Corridor Attributes: Number of Lanes:
4 each direction Washington to Brighton 3 each direction Brighton to Chambers 2 each direction Chambers to E-470
Outside Shoulder Width: 10
Inside Shoulder Width: $2^{\prime}$
Median Width: Varies $0^{\prime}$ to $60^{\prime}$
Terrain: Level-Rolling
Toll Collection System: Electronic Toll Collection (ETC)

Open-Road Tolling

## LEGEND

4-Lane Express Tollway
4-Lane Elevated Express Tollway
Transition to General Purpose (GP) Lanes

## , Access

Slip Toll Access: EB Ent.NB Exit
4 Slip Toll Access: WB Ent/EB Ext
D Direct Ramp Toll Access: EB Ent/WB Exit
Direct Ramp Toll Access: WB Ent/EB Exit

with different improvement types. From I-25 to just east of I-270, the section would have three general purpose lanes in each direction, the majority of which is on elevated structure, and two express toll lanes each direction on elevated structure, located adjacent to the existing I-70 alignment on the north side. From just east of I-270 to Chambers, I-70 would have three general purpose lanes and two express toll lanes in each direction located at-grade. Within this section the express toll lanes would be located in the median of existing I-70 and would be separated from the general purpose lanes by a concrete barrier. It was assumed that the existing general purpose lanes would need to be reconstructed between I270 and Chambers because the current median width is not sufficient to add express toll lanes in the median without impacting the general purpose lanes.

The existing right-of-way along the eastern portion of the project from I270 to Chambers was assumed to be adequate and no new right-of-way was assumed to be purchased. However, right-of-way from I-25 to I-270 was assumed to be acquired since the section was located on elevated structure to the north of the existing I-70 alignment. The proposed section shown in Figure 5-7 provides the widths for lanes, median treatments and shoulders assumed for the improved roadway.

For the elevated roadway structure, access points were assumed to be needed at Colorado Boulevard, Quebec and I-270 through direct drop down ramps. For the section at-grade, no new interchanges were assumed for the corridor, but some existing interchanges required ramp or bridge modifications in order to construct the express toll lanes. Most of the mainline bridges in this section were assumed to be widened to accommodate the express toll lanes. As shown on Figure 5-7, access would be provided through direct connections either as direct drop down ramps on the elevated structure or via dedicated flyover ramps at I-225 and I-270 to and from the express toll lanes on I-70 to the general purpose lanes on I-225 and I-270.

Information from the ongoing I-70 Environmental Impact Statement was used to develop the cost estimate for the project.

## I-70 East Scenario 3

The I-70 East Scenario 3 project is located between Colorado and Chambers. The project is approximately 6 miles and would not have an elevated structure as was assumed in Scenarios 1 and 2. From just east of Colorado to Chambers, I-70 would have three general purpose lanes and two express toll lanes in each direction located at-grade. Within this section the express toll lanes would be located in the median of existing I-

Project Limits: $1-25$ (begin Log 274) to Chambers (end $\log 282$ )
Project Length: 8 Miles
Project Type: Express Toll Lanes
Description: Tolling new capacity

Improvement Scenario: Add 2 express toll lanes each direction.
Existing Corridor Attributes: Number of Lanes:
4 each direction Washington to Brighton
3 each direction Brighton to Chambers
Outside Shoulder Width: $10^{\prime}$
Inside Shoulder Width: $2^{\prime}$
Median Width: Varies $0^{\prime}$ to $60^{\prime}$
Terrain: Level-Rolling


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70 and would be separated from the general purpose lanes by a concrete barrier. It was assumed that the existing general purpose lanes would need to be reconstructed between Colorado and Chambers because the current median width is not sufficient to add express toll lanes in the median without impacting the general purpose lanes.

The existing right-of-way was assumed to be adequate and no new right-of-way was assumed to be purchased. The proposed section shown in Figure 5-8 provides the widths for lanes, median treatments and shoulders assumed for the improved roadway. No new interchanges were assumed for the corridor, but some existing interchanges required ramp or bridge modifications in order to construct the express toll lanes. Most of the mainline bridges in this section were assumed to be widened to accommodate the express toll lanes. As shown on Figure 5-8, access would be provided at the beginning and ending of the toll facility and through direct connections via dedicated flyover ramps at I-225 and I-270 to and from the express toll lanes on I-70 to the general purpose lanes on I-225 and I-270.

Information from the ongoing I-70 Environmental Impact Statement was used to develop the cost estimate for the project.

## U.S. 36 Scenario 1

As shown in Figure 5-9, the US 36 project extends from Foothills Parkway near the city limits of Boulder to the eastern terminus at I-25. The project is approximately 18 miles long and is subdivided into three sections with different improvement types. From Foothills Parkway to McCaslin Boulevard, the section would have two general purpose lanes and one express toll lane each direction. From McCaslin Boulevard to Pecos, US 36 would have two general purpose lanes and two express toll lanes in each direction and from Pecos to I-25, the project includes converting the existing one-lane reversible high-occupancy vehicle (HOV) facility to a two-lane reversible high-occupancy toll (HOT) facility. The section would then have two general purpose lanes in each direction.

In all three sections the express toll lanes would be located in the median of the existing roadway. From Foothills Parkway to I-25, the express toll lanes would be separated from the general purpose lanes by a concrete barrier. It was assumed that the majority of the existing general purpose lanes would need to be reconstructed because the current median width is not sufficient to add express toll lanes in the median without impacting the general purpose lanes. The existing right-of-way along the eastern portion of the project was approximately 300 feet, but the right-of-way for the majority of the corridor was within a range of 150 to 200 feet. In order to

Project Limits: Colorado (begin 276) to Chambers (end 282)

## Project Length: 6 Miles

Project Type: Express Toll Lanes
Description: Tolling new capacity

## Improvement Scenario: Add 2 express toll lanes each direction

Existing Corridor Attributes: Number of Lanes: 3 each direction Outside Shoulder Width: 10 Inside Shoulder Width: $2^{\prime}$ Median Width: Varies $0^{\prime}$ to $60^{\prime}$ Terrain: Level-Rolling

Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling


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Project Limits: $1-25$ (begin Log 57 ) to Boulder, Foothills Parkway (end Log 39)
Project Length: 18 Miles
Project Type: Managed Lanes and HOT Lanes
Description: Tolling new capacity, convert HOV to HOT lanes
Improvement Scenarios: - Add 1 express toll lane each direction
Cherry Vale Rd. to McCaslin Blvd.

- Add 2 express toll lanes each direction

McCaslin Blvd. to Pecos St

- Convert existing 1-lane reversible HOV to 2-lane Reversible HOT Pecos St. to I-25.

Existing Corridor Attributes: Number of Lanes:
-2 each direction (Cherry Vale to Pecos)
3 each direction (Pecos to l-25)
Outside Shoulder Width: $10^{\prime}$
Inside Shoulder Width: $4^{\prime}$
Median Width: 8
Terrain: Rolling
Toll Collection System: Electronic Toll Collection (ETC) / Open-Road Tolling


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construct the express toll lanes, it was assumed that right-of-way would need to be purchased at several locations along the corridor. The majority of the right-of-way would occur in the section from Broomfield to I-25.
The proposed section shown in Figure 5-9 provides the widths for lanes, median treatments and shoulders assumed for the improved roadway. Most of the existing interchanges required ramp or bridge modifications in order to construct the express toll lanes. The Broomfield Interchange was estimated to be the most costly interchange improvement for the corridor at approximately $\$ 173$ million.

As shown on Figure 5-9, the majority of the express toll lane access along the corridor would be provided through slip toll access between existing interchanges. Direct connections to and from the HOT facility on US 36 to the I-25 HOV facility and the I-25 general purpose lanes via dedicated flyover ramps were assumed at the I-25 interchange.

Information from the ongoing US 36 Environmental Impact Statement was used to develop the cost estimate for the project. In order to be consistent with the ongoing study, the contingency was adjusted from 20 percent to 30 percent and a CDOT Force Account Miscellaneous was added to the overall capital cost estimate.

## I-225 Scenario 1

The I-225 project spans approximately 8 miles from I-70 to S.H. 83. I-225 would have two express toll lanes in each direction and two general purpose lanes in each direction. The express toll lanes would be located in the median of the existing roadway and would be separated from the general purpose lanes by a concrete barrier. The section of the corridor from Parker Road to $6^{\text {th }}$ Avenue has received environmental clearance for constructing six general purpose lanes and is included in the current TIP program; however the project has not been implemented due to a lack of funding. For the purposes of this study it was assumed that the improvements identified in the 2000 Environmental Assessment would be implemented in conjunction with the express toll lanes with the exception that only four general purpose lanes would be reconstructed instead of six as originally planned.

For the section of I-225 from $6^{\text {th }}$ Avenue to I-70, it was assumed that widening could occur on the outside to provide the additional width required for express toll lanes. The existing right-of-way width is approximately 300 feet and was determined to be adequate; therefore no new right-of-way was assumed to be purchased.


## Project Limits: S.H. 83 (begin Log 4) to I-70 (end Log 12)

Project Length: 8 Miles
Project Type: Express Toll Lanes
Description: Tolling new capacity
Improvement Scenario: Add 2 express toll lanes each direction.
Existing Corridor Attributes: Number of Lanes: 3 each direction (1-70 to 6th Ave.) 2 each direction (6th Ave. to SH 83)
Outside Shoulder Width: $\mathbf{1 0}^{\prime}$
Inside Shoulder Width: 4'
Median Width: 60
Terrain: Level
Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling



The proposed section shown in Figure 5-10 provides the widths for lanes, median treatments and shoulders assumed for the improved roadway. No new interchanges were assumed for the corridor, but some existing interchanges required ramp or bridge modifications in order to construct the express toll lanes.

As shown on Figure 5-10, the majority of the express toll lane access along the corridor would be provided through slip toll access between existing interchanges, except for at the beginning and ending sections of the express toll facility, which would have direct access and a transition area, similar to the example shown in Figure 5-1.

## I-270 Scenario 1

The I-270 Scenario 1 is shown in Figure 5-11. The project spans approximately 5 miles between I-25 and I-70. I-270 would have two general purpose lanes and two express toll lanes in each direction. The express toll lanes would be located in the median of the existing roadway and would be separated from the general purpose lanes by a concrete barrier. It was assumed that the existing general purpose lanes would need to be reconstructed because the current median width is not sufficient to add two express toll lanes each direction. The existing right-of-way width was determined to be adequate in most locations; however, new right-ofway was included in the overall roadway construction cost at spot locations along the corridor. The proposed section provides the widths for lanes, median treatments and shoulders assumed for the improved roadway.

No new interchanges were assumed for the corridor, but some existing interchanges required ramp or bridge modifications in order to construct the express toll lanes. Most of the mainline bridges were assumed to be widened to accommodate the express toll lanes.

The majority of the express toll lane access along the corridor would be provided through slip toll access between existing interchanges, except for at the beginning and ending sections of the express toll facility, which would have direct access and a transition area, similar to the example shown in Figure 5-1.

In order to be consistent with internal CDOT preliminary cost estimates for I-270 the contingency was adjusted from 20 percent to 30 percent and a CDOT Force Account Miscellaneous was added to the overall capital cost estimate.


Improvement Scenario: Add 2 express toll lanes each direction.
Existing Corridor Attributes: Number of Lanes: 2 each direction Outside Shoulder Width: 10 Inside Shoulder Width: $8^{\prime}$ Median Width: Varies 50 ' to 52 Terrain: Rolling

Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling

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## C-470 Scenarios 1 and 1A

The C-470 project spans approximately 14 miles from just east of I-25, connecting to the terminus of the existing E-470 Tollway, to Kipling Parkway. From just east of I-25 to east of Wadsworth Boulevard, C-470 would have two general purpose lanes and two express toll lanes in each direction. The express toll lanes would be located in the median of the existing roadway and would be separated from the general purpose lanes by a concrete barrier. From east of Wadsworth Boulevard to Kipling, C470 would have two general purpose lanes and one express toll lane in each direction. The express toll lanes would be located in the median of the existing roadway and would be separated from the general purpose lanes by a four-foot buffer. It was assumed that the existing general purpose lanes would not be reconstructed. The existing right-of-way width was determined to be adequate in most locations (ranging between 250 to 300 feet); however, new right-of-way was included in the overall roadway construction cost at spot locations along the corridor. The proposed section shown in Figure 5-12 provides the widths for lanes, median treatments and shoulders assumed for the improved roadway.

Some existing interchanges required ramp or bridge modifications in order to construct the express toll lanes. Most of the mainline bridges were assumed to be widened to accommodate the express toll lanes. In addition, a new t-ramp connection exclusively for the express toll lanes was assumed to be constructed at Colorado Boulevard.

As shown on Figure 5-12, the majority of the express toll lane access along the corridor would be provided through slip toll access between existing interchanges. Direct connections to the express toll lanes via dedicated flyover ramps were assumed at I-25, as well as direct connections at Colorado Boulevard and Quebec Street.

Information from the ongoing C-470 Corridor Environmental Assessment was used to develop the cost estimates and express toll lane access locations for the project.

## C-470 Scenarios 2 And 2A

The C-470 Scenario 2 project limits are from just east of I-25, connecting to the terminus of the existing E-470 Tollway, to I-70. The project is approximately 26 miles long. Scenario 2 would have two general purpose lanes and two express toll lanes in each direction, including the section from Platte Canyon Road to Kipling Parkway. The express toll lanes would be located in the median of the existing roadway and would be separated from the general purpose lanes by a concrete barrier. It was assumed that the existing general purpose lanes would not be


Project Limits: Kipling (begin $\log 12$ ) to $1-25$ (end Log 26 ) Project Length: 14 Miles

Project Type: Express Toll Lanes Description: Tolling new capacity

Improvement Scenarios:
Add 2 express toll lanes each direction just west of Platte Canyon to $1-25$. Add 1 express toll lane each direction Kipling to just west of Platte Canyon.

Existing Corridor Attributes: 2 each direction
Outside Shoulder Width: $\mathbf{1 0}^{\prime}$ Inside Shoulder Width: 4' Median Width: Varies $20^{\prime}$ to $40^{\prime}$ Terrain: Rolling

Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling

reconstructed. The existing right-of-way width was determined to be adequate in most locations (ranging between 250 to 400 feet); however, new right-of-way was included in the overall roadway construction cost at spot locations along the corridor.

The section from Morrison Road to I-70 was assumed to only need one express toll lane constructed each direction, with modifications to the existing general purpose lanes and shoulders to accommodate the express toll lanes in the median. This section had been recently widened to six general purpose lanes and while it is recognized that existing capacity cannot be tolled under Colorado law, this was assumed for analysis purposes. The proposed section shown in Figure 5-13 provides the widths for lanes, median treatments and shoulders assumed for the improved roadway.

Some existing interchanges required ramp or bridge modifications in order to construct the express toll lanes. Most of the mainline bridges were assumed to be widened to accommodate the express toll lanes. In addition, a new t-ramp connection exclusively for the express toll lanes was assumed to be constructed at Colorado Boulevard.

As shown on Figure 5-13, the majority of the express toll lane access along the corridor would be provided through slip toll access between existing interchanges. Direct connections to the express toll lanes via dedicated flyover ramps were assumed at the I-25, as well as direct connections at Colorado Boulevard and Quebec Street.

Information from the ongoing C-470 Corridor Environmental Assessment was used to develop the cost estimates and express toll lane access locations for the project.

## Northwest Corridor Scenario 1

The Northwest Corridor Scenario 1 project consists of developing a new roadway corridor between U.S. 36 and C-470, connecting it to the existing Northwest Parkway Tollway and completing the outer beltway around Denver. The new corridor would be approximately 24 miles long and includes a four-lane roadway on new alignment. Most of the project was assumed to be on new right-of-way; however, the section from the interchange at S.H. 93 south to C-470 follows the existing S.H. 93/U.S. 6 alignment and will require additional right-of-way at various points along the corridor. The proposed section shown in Figure 5-14 provides the widths for lanes, median treatments and shoulders assumed for the new roadway.


## Toll Collection System: Electronic Toll Collection (ETC) <br> Open-Road Tolling

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LEGEND
4-Lane Express Talway
Transition to General Purpose (GP) Lanes
Tollway Access
- Slip Toll Access: Serves AlL Directions
    Slip Toll Accoss: EB EntNB Exit
    Slip Toll Access: WB Ent/EB Exil
    Slip Toll Access:WB ExitUEB Ex
    Direct Ramp Toll Access: Serves ALLLDirections
    Direct Ramp Toll Access: EB Ent/WB Exit
    Direct Ramp Toll Access:WB Ent/EB Exit


Project Limits: C-470 to NW Parkway/US 36
Project Length: 24 Miles
Project Type: New Toll Road
Description: Tolling new capacity
Improvement Scenario: Build two new toll lanes in each
direction on new alignment.
Planned Corridor Attributes: Number of Lanes: 2 each direction Outside Shoulder Width: 10'
nside Shoulder Width: \(4^{\prime}\)
Median Width: 64
Terrain: Rolling
Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling


Engineers

New interchanges were assumed at nine locations along the corridor at major interstate, highway and arterial crossings. All capacity would be tolled through ETC/ Open Road Tolling so additional tollway access points and toll plazas to enter and exit the toll road were not required.
Information from the ongoing Northwest Corridor EIS was used to develop the cost estimates.

\section*{Northwest Corridor Scenario 2}

The Northwest Corridor Scenario 2 project follows the same alignment as Scenario 1, however, the tolled section of the corridor would extend from S.H. 128 to S.H 58. The new corridor would be approximately 14 miles long and includes a four-lane roadway on new alignment. Most of the project was assumed to be on new right-of-way, however, the section from the interchange at S.H. 93 south to S.H. 58 follows the existing S.H. 93/U.S. 6 alignment and will require additional right-of-way at various points along the corridor. The proposed section shown in Figure 5-15 provides the widths for lanes, median treatments and shoulders assumed for the new roadway.

New interchanges were assumed at five locations along the corridor at major highway and arterial crossings. All capacity would be tolled through ETC/ Open Road Tolling so additional tollway access points and toll plazas to enter and exit the toll road was not required. Information from the ongoing Northwest Corridor EIS was used to develop the cost estimates.

Information from the ongoing Northwest Corridor EIS was used to develop the cost estimates.

\section*{U.S. 287 Bypass Scenario 1}

The U.S. 287 Bypass project consists of developing a new corridor, bypassing Fort Collins near S.H. 14 and U.S. 287. The new bypass would be approximately 12 miles long, located between existing U.S. 287, near Livermore, and I-25. The new bypass would be a four-lane roadway on new alignment. The entire project was assumed to be on new right-of-way. The proposed section shown in Figure 5-16 provides the widths for lanes, median treatments and shoulders assumed for the new roadway.

New interchanges were assumed at the intersection of U.S. 287 and the new bypass and at a median point along the corridor. The existing interchange with I-25 and CR 70 would be utilized by the new bypass, but was assumed to require ramp modifications in order to construct the new toll road.


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Project Limits: US 287 to I-25 Connector
Project Length: 12 Miles
Project Type: New Toll Road
Description: Tolling new capacity

Improvement Scenario: Build 2 toll lanes each direction on new alignment.

Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling

Planned Corridor Attributes: Number of Lanes: 2 each direction Outside Shoulder Width: 10 Outside Shoulder Width: \({ }^{\prime}\) Median Width: \(60^{\circ}\) Terrain: Rolling

U.S. 287 Corridor
(4 Lane Express Tollway)

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All capacity would be tolled through ETC/ Open Road Tolling so additional tollway access points and toll plazas to enter and exit the toll road were not required.

\section*{Front Range Scenario 1}

The Front Range Scenario 1 project consists of developing a new roadway corridor between I-25 north of Fort Collins to I-25 south of Pueblo. The new corridor would be approximately 194 miles long. The new toll road would be a four-lane roadway on new alignment. The entire project was assumed to be on new right-of-way. The proposed section shown in Figure 5-17 provides the widths for lanes, median treatments and shoulders assumed for the new roadway.

New interchanges were assumed at 11 locations along the corridor at major interstate and highway crossings. All capacity would be tolled through ETC/ Open Road Tolling so additional tollway access points and toll plazas to enter and exit the toll road were not required.

\section*{Front Range Scenario 2}

The Front Range Scenario 2 project is similar to Scenario 1, except that it consists of developing a new roadway corridor between I-25 north of Fort Collins to I-25 north of Pueblo. The new corridor would be approximately 169 miles long. The new toll road would be a four-lane roadway on new alignment. The entire project was assumed to be on new right-of-way. The proposed section shown in Figure 5-18 provides the widths for lanes, median treatments and shoulders assumed for the new roadway.

New interchanges were assumed at 10 locations along the corridor at major interstate and highway crossings. All capacity would be tolled through ETC/ Open Road Tolling so additional tollway access points and toll plazas to enter and exit the toll road were not required.

\section*{Powers Boulevard Scenario 1}

The Powers Boulevard Scenario 1 project consists of developing a new roadway corridor between I-25 north of Colorado Springs (Northgate Rd.) and Woodmen Road. The new corridor would be constructed as a fourlane roadway and the majority of the project would be located on new alignment. The corridor is approximately nine miles long. Most of the project required new right-of-way; however, a portion of the corridor followed the existing Powers Boulevard alignment. The project would relocate existing general purpose lanes to one-way, two-lane frontage roads located on both sides of the proposed toll road. These frontage roads would include signalized intersections. The proposed section shown in

Project Limits: \(\mathrm{I}-25 \mathrm{~N}\) (at Fort Collins) to \(\mathrm{I}-25 \mathrm{~S}\) ( South of Pueblo)
Project Length: 194 Miles
Project Type: New 4-Lane Toll Road
Description: Tolling new capacity
Improvement Scenario: Build two toll lanes in each
direction on new alignment.
Planned Corridor Attributes: Number of Lanes: 2 each direction Outside Shoulder Width: 10 Inside Shoulder Width: \(\mathbf{4}^{1}\) Median Width: 60 Terrain: Rolling
Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling


Project Limits: \(1-25 \mathrm{~N}\) (at Fort Collins) to \(\mathrm{I}-25 \mathrm{~S}\) (North of Pueblo)
Project Length: 169 Miles
Project Type: New 4-Lane Toll Road
Description: Tolling new capacity
Improvement Scenario: Build two toll lanes in each direction on new alignment.
Planned Corridor Attributes: Number of Lanes: 2 each direction Outside Shoulder Width: 10
Iside Shoulder Wid
Terrain: Rolling
Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling


Figure 5-19 provides the widths for lanes, median treatments and shoulders assumed for the new roadway.

New interchanges were assumed at eight locations along the corridor at major interstate highway and arterial crossings. All capacity would be tolled through ETC/ Open Road Tolling so additional tollway access points and toll plazas to enter and exit the toll road were not required.

Information from the ongoing Powers Corridor Improvement Project was used to develop the cost estimate for the project.

\section*{Powers Boulevard Scenario 2}

The Powers Boulevard Scenario 2 project consists of developing a new roadway corridor from I-25 north of Colorado Springs (Northgate Rd.) to Drennan Road. The corridor is approximately 21 miles long. The new corridor would be constructed as a four-lane toll road and some portions of the project would be located on new alignment. New right-of-way is required for the north segment of the project (I-25N to Woodmen); however, the majority of the corridor followed the existing Powers Boulevard alignment and will require additional right-of-way at various points along the corridor. The project would relocate the existing general purpose lanes to one-way, two-lane or three-lane frontage roads located on both sides of the proposed toll road. These frontage roads would include signalized intersections. The proposed section shown in Figure 5-20 provides the widths for lanes, median treatments and shoulders assumed for the new roadway. It also shows the limits of the frontage roads.

New interchanges were assumed at 19 locations along the corridor at major interstate, highway and arterial crossings. All capacity would be tolled through ETC/ Open Road Tolling so additional tollway access points and toll plazas to enter and exit the toll road were not required.

Information from the ongoing Powers Corridor Improvement Project was used to develop the cost estimate for the project.

\section*{Powers Boulevard Scenario 3}

The Powers Boulevard Scenario 3 project consists of developing a new roadway corridor from I-25 north of Colorado Springs (Northgate Rd.) to south of Fontaine Boulevard. The corridor is approximately 27 miles long. The new corridor would be constructed as a four-lane toll road and some portions of the project would be located on new alignment. New right-of-way is required for the north segment of the project (I-25N to Woodmen); however, the majority of the corridor followed the existing Powers Boulevard alignment and will require additional right-of-way at


Project Limits: \(1-25 \mathrm{~N}\) (Northgate Rd.) to Woodmen Rd.
Project Length: 9 Miles
Project Type: New 4-Lane Toll Road
Description: Tolling new capacity
Improvement Scenario: Build two express lanes in each
direction on new alignment.
Planned Corridor Attributes: Number of Lanes (Mainline): 2 each direction
Number of Lanes (Frontage Roads): Varies 2 to 3 lanes each direction utside Shoulder Width: 10
nside Shoulder Width: 10
Median Width: 22'
Terrain: Rolling
Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling


Powers Blvd. Corridor

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Project Limits: \(1-25 \mathrm{~N}\) (Northgate Rd.) to Drennan
Project Length: 21 Miles
Project Type: New 4-Lane Toll Road
Description: Tolling new capacity
Improvement Scenario: Build two express lanes in each
direction on new alignment.
Planned Corridor Attributes: Number of Lanes (Mainline): 2 each direction
Number of Lanes (Frontage Roads): Varies 2 to 3 lanes each direction Outside Shoulder Width: 10'
Inside Shoulder Width: \(10^{\prime}\)
Median Width: \(2^{\prime}\)
Terrain: Rolling
Toll Collection System: Electronic Toll Collection (ETC)
Open-Road Tolling


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CTE Preliminary Traffic And Revenue Study
Wilbur Smith Associates
various points along the corridor. The project would relocate the existing general purpose lanes to one-way, two-lane or three-lane frontage roads located on both sides of the proposed toll road. These frontage roads would include signalized intersections. The proposed section shown in Figure 5-21 provides the widths for lanes, median treatments and shoulders assumed for the new roadway. It also shows the limits of the frontage roads.

New interchanges were assumed at 24 locations along the corridor at major interstate, highway and arterial crossings. All capacity would be tolled through ETC/ Open Road Tolling so additional tollway access points and toll plazas to enter and exit the toll road were not required.

Information from the ongoing Powers Corridor Improvement Project was used to develop the cost estimate for the project.

\section*{Powers Boulevard Scenario 4}

The Powers Boulevard Scenario 4 project consists of developing a new toll road from I-25 north of Colorado Springs (Northgate Rd.) to Woodmen Road (Scenario 1), reconstructing Powers Boulevard from Woodmen Road to Drennan Road as a six lane freeway as currently proposed in the ongoing Powers Corridor Improvement Project, and constructing a new toll road that generally follows the existing Drennan Road alignment from the Powers Boulevard/Drennan Road Interchange to I-25 just north of Academy Boulevard. The section from I-25 (Northgate) to Woodmen Road would be the same as that described in Scenario 1. The new corridor west of Powers/Drennan Interchange would be constructed as a four-lane toll road and some portions of the project would be located on new alignment. The corridor is approximately 12 miles long. New right-of-way is required at the north and south ends of the project; however, the majority of the corridor followed the existing Powers Boulevard alignment. The proposed section shown in Figure 5-22 provides the widths for lanes, median treatments and shoulders assumed for the new roadway. It also shows the limits of the frontage roads.

New interchanges were assumed at 10 locations along the corridor at major interstate, highway and arterial crossings. All capacity would be tolled through ETC/ Open Road Tolling so additional tollway access points and toll plazas to enter and exit the toll road were not required.

Information from the ongoing Powers Corridor Improvement Project was used to develop the cost estimate for the project.

\(\begin{aligned} \text { Project Limits: } & \\ & \text { I-25N (Northgate Rd.) to } \\ & \text { New Arterial East of l-25S }\end{aligned}\)
Project Length: 27 Miles
Project Type: New 4-Lane Toll Road
Description: Tolling new capacity
Improvement Scenario: Build two express lanes in each
direction on new alignment
Planned Corridor Attributes: Number of Lanes (Mainline): 2 each direction
Number of Lanes (Frontage Roads): Varies 2 to 3 lanes each direction
Outside Shoulder Width: 10
Inside Shoulder Width: 10
Median Width: 22'
Terrain: Rolling
Toll Collection System: Electronic Toll Collection (ETC)
Open-Road Tolling


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Project Limits: \(1-25 \mathrm{~N}\) (Northgate Rd.) to Woodmen Road and \(1-25 \mathrm{~S}\) to Powers/Drennan interchange.
Project Length: 12 Miles
Project Type: New 4-Lane Toll Road
Description: Tolling new capacity
Improvement Scenario: Build two express lanes in each direction on new alignment.

Planned Corridor Attributes: Number of Lanes (Mainline): 2 each direction
Number of Lanes (Frontage Roads): Varies 2 to 3 lanes each direction Outside Shoulder Width: \(10^{\prime}\) Outside Shoulder Width: \({ }^{\prime}\)
Inside Shoulder Width: 22'
Terrain: Rolling
Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling


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\section*{Banning-Lewis Parkway Scenarios 1 and 2}

The Banning-Lewis Parkway project consists of developing a new roadway corridor between I-25 north of Colorado Springs (Northgate Road) to I-25 south (Academy Boulevard). The new corridor would be approximately 31 miles long and includes a four-lane roadway on new alignment. The entire project was assumed to be on new right-of-way, however, right-of-way within the Banning Lewis Ranch property was assumed to be dedicated per the annexation agreement with the City of Colorado Springs. The proposed section shown in Figure 5-23 provides the widths for lanes, median treatments and shoulders assumed for the new roadway.

New interchanges were assumed at 11 locations along the corridor at major interstate, highway and arterial crossings. All capacity would be tolled through ETC/ Open Road Tolling so additional tollway access points and toll plazas to enter and exit the toll road were not required.

Information from the Banning Lewis Conceptual Design Report dated September 1998 and the ongoing Springs Toll Road Study were used to develop the cost estimates.

\section*{I-70 Mountain Corridor Scenario 1}

The I-70 Mountain Corridor Scenario 1 project spans approximately 34 miles from the Eisenhower Tunnels to Floyd Hill. Within this scenario, I70 would have two general purpose lanes each direction and two reversible express toll lanes located in the median of the existing roadway, separated from the general purpose lanes by a concrete barrier. In addition, a new two-lane tunnel bore would be constructed at both the Eisenhower and Twin Tunnels. It was assumed that the majority of the existing general purpose lanes would need to be reconstructed because the current median width is not sufficient to add two reversible express toll lanes in the median. The existing right-of-way width was determined to be adequate in most locations (approximately 300 feet); however, new right-of-way was included in the overall roadway construction cost at spot locations near Idaho Springs, between Silverplume and Georgetown, and at the Eisenhower and Twin Tunnels for the new tunnel bores.

The proposed sections shown in Figure 5-24 provide the widths for lanes, median treatments, tunnel bores and shoulders assumed for the improved roadway.

No new interchanges were assumed for the corridor, but some existing interchanges required ramp or bridge modifications in order to construct the reversible express toll lanes. As shown on Figure 5-24, access to the


Banning-Lewis Parkway Corridor
(4-Lane Express Tollway)

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Project Limits: West of Eisenhower Tunnel to Floyd Hill

\section*{Project Length: 34 Miles}

Project Type: Reversible Lanes
Description: Tolling new roadway capacity Tolling only reversible tunnel bore

Improvement Scenario: Add 2 reversible express toll lanes.
Existing Corridor Attributes: Number of Lanes: 2 each direction Outside Shoulder Width: 10 Inside Shoulder Width: 4 Median Width: Varies \(4^{\prime}\) to \(>60^{\prime}\) Terrain: Mountainous

Toll Collection System: Electronic Toll Collection (ETC) Open-Road Tolling

1.70 Mountain Corridor
(2.Lane Reversible-S Structured C Configuration in Idation
2.Lane Reversible- Structured Configuration in Idaho Springs)


1-70 Mountain Corridor

NOTE: Some access points are closed weekly or seasonally, depending on
reversible lanes direction.


Eisenhower/Johnson Memo

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reversible express toll lanes would be limited and would involve slip toll access, except for at the beginning and ending sections of the express toll facility, which would have direct access and a transition area, similar to the example shown in Figure 5-1.

Information from the ongoing I-70 Mountain Corridor Environmental Impact Statement was used to develop the cost estimate for the project.

\section*{I-70 Mountain Corridor Scenario 2}

The I-70 Mountain Corridor Scenario 2 project spans approximately 34 miles from the Eisenhower Tunnels to Floyd Hill. I-70 currently has two general purpose lanes each direction and one additional lane would be added each direction. In addition, a new two-lane tunnel bore would be constructed at the Eisenhower Tunnels and a new three-lane tunnel bore at the Twin Tunnels. The existing right-of-way width was determined to be adequate in most locations (approximately 300 feet); however, new right-of-way was included in the overall roadway construction cost at spot locations near Idaho Springs, between Silverplume and Georgetown, and at the Eisenhower and Twin Tunnels for the new tunnel bores.

The proposed sections shown in Figure 5-25 provide the widths for lanes, median treatments, tunnel bores and shoulders assumed for the improved roadway. No new interchanges were assumed for the corridor, but some existing interchanges required ramp or bridge modifications in order to construct the express toll lanes. As shown on Figure 5-25, all capacity would be tolled so new access points would need to be constructed for the facility.

Information from the ongoing I-70 Mountain Corridor Environmental Impact Statement was used to develop the cost estimate for the project.

\section*{I-70 Mountain Corridor Scenarios 3, 3A, 3B}

The I-70 Mountain Corridor Scenarios 3, 3A and 3B consist of constructing a new two-lane tunnel bore at the Eisenhower Tunnels and a new three-lane tunnel bore at the Twin Tunnels with one-half mile approach roadways on each side of the tunnels. The project length is approximately five miles with both tunnels combined. New right-of-way was assumed for construction of the new tunnel bores.

The proposed sections shown in Figure 5-26 provides the widths for lanes, median treatments, tunnel bores and shoulders assumed for the new tunnels and approach roadways. All capacity at all tunnel bores would be tolled.

Project Limits: Eisenhower Tunnel to Floyd Hill
Project Length: 34 Miles
Project Type: New Toll Lanes
Description: Tolling all capacity

Improvement Scenario: Add 1 Lane each direction; toll all lanes Existing Corridor Attributes: Number of Lanes: 2 each direction Outside Shoulder Width: \({ }^{10}\) Inside Shoulder Width: \(4^{\prime}\) Median Width: Varies \(4^{\prime}\) to \(>60^{\prime}\) Terrain: Mountainous
Toll Collection System: Closed Barrier Toll Collection with Manual and ETC

1.70 Mountain Corridor

I. 70 Mountain Corridor
Structured Configuratoon in lasho Springs

Eisenhower/Johnson Memorial Tunnel


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Project Limits: Eisenhower Tunnel and Twin Tunnel
Project Length: 5 Miles
Project Type: New Tunnel Bores
Description: Tolling all capacity

Improvement Scenario: - Add new 2-lane tunnel bore Eisenhower Tunnels
Add new 2-ane tunnel bore Eisenhower
Existing Corridor Attributes: Number of Lanes: 2 each direction
Outside Shoulder Width: 10
Outside Shoulder Width: \(4^{\prime}\)
Median Width: Varies \(4^{\prime}\) to \(>60^{\prime}\)
Terrain: Mountainous
Toll Collection System: Closed Barrier Toll Collection with Manual and ETC


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Information from the ongoing I-70 Mountain Corridor Environmental Impact Statement was used to develop the cost estimate for the project.

Table 5-1 shows the roadway capital costs for each project in 2004 dollars.

\section*{ESTIMATED TOLL COLLECTION CAPITAL COSTS}

The estimated toll system capital costs are shown in Table 5-2. The capital costs are typically subdivided into the following categories for each project:
- Structures - The capital cost for overhead equipment mounting structures and roadside structures for housing equipment.
- Communications - The capital cost for installing a fiber optic communication backbone interconnecting dynamic and changeable signs and tolling points with the local carriers network interconnection.
- Power - The capital cost to install electrical and power backup to the roadside structure housing transaction processing and communication equipment.
- Electronic Toll Collection - The capital cost to furnish and install the components of the ETC subsystem to record transactions that are used to build trips and charge accounts for facility usage.
- Vehicle Detection and Violation Trigger - The capital cost to furnish and install the detection and triggering components of the violation enforcement system that is implemented to assure the integrity of the System by issuing citations to users who fail to obtain a valid transponder to use the facility.
- Violation Enforcement System - The capital cost to furnish and install the components of the of the violation enforcement system that is implemented to assure the integrity of the System by issuing citations to users who fail to obtain a valid transponder to use the facility.
- Lane Processing - The capital cost to furnish and install the lane processing equipment used to identify valid transactions based on the transponder ID read from the vehicle, coordinate updates to the list of transponder ID, and build transaction records that are subsequently used to build trips by the customer service center server.
- Vehicle Access Control - The capital cost to furnish and install overhead dynamic message and combined fixed, static and changeable message signs for informing users regarding the approaching express lane facility and the trip charges to various destinations.
- Host Processing - The capital cost for the host computer system that in located at the Agency's leased office space and used to process the

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\begin{tabular}{|c|c|c|c|c|}
\hline \multicolumn{5}{|l|}{\begin{tabular}{l}
Table 5-1 \\
Summary of Roadway Capital Cost Estimates Second-Tier Candidate Toll Projects
\end{tabular}} \\
\hline Location & Limits & Length (miles) & \begin{tabular}{c} 
Roadway \\
\begin{tabular}{c} 
Capital Costs \\
\((000)\)
\end{tabular} \\
\hline
\end{tabular} & \begin{tabular}{c} 
Roadway \\
Cost per Mile \\
\((000)\) \\
\hline
\end{tabular} \\
\hline I-25 Express Toll Lanes Scenario 1 & U.S. 36 to S.H. 66 & 26 & \$ 299,200 & 11,508 \\
\hline I-25 Express Toll Lanes Scenario 2 & U.S. 36 to S.H. 7 & 12 & 225,800 & 18,817 \\
\hline I-70 East Express Toll Lanes Scenario 1 & I-25 to E-470 & 12 & 648,000 & 54,000 \\
\hline I-70 East Express Toll Lanes Scenario 2 & I-25 to Chambers Road & 8 & 555,200 & 69,400 \\
\hline I-70 East Express Toll Lanes Scenario 3 & Colorado to Chambers Road & 6 & 258,600 & 43,100 \\
\hline U.S. 36 Express Toll Lanes & I-25 to Foothills Parkway & 18 & 1,206,100 & 67,006 \\
\hline I-225 Express Toll Lanes & I-70 to S.H. 83 & 8 & 171,600 & 21,450 \\
\hline I-270 Express Toll Lanes & I-25 to I-70 & 5 & 205,700 & 38,093 \\
\hline C-470 Express Toll Lanes Scenario 1, 1A & I-25 to Kipling & 14 & 314,200 & 22,443 \\
\hline C-470 Express Toll Lanes Scenario 2, 2A & I-25 to I-70 & 26 & 514,000 & 19,769 \\
\hline Northwest Corridor Toll Road Scenario 1 & C-470 to NW Parkway/US 36 & 24 & 852,600 & 35,525 \\
\hline Northwest Corridor Toll Road Scenario 2 & SH 128 to SH 58 & 14 & 319,200 & 22,800 \\
\hline I-70 West Toll Road Scenario 1 & West of Eisenhower Tunnel to Floyd Hill & 34 & 2,603,500 & 76,574 \\
\hline I-70 West Toll Road Scenario 2 & Eisenhower Tunnel to Floyd Hill & 34 & 2,480,300 & 72,950 \\
\hline I-70 West Toll Road Scenario 3, 3A, 3B & Eisenhower Tunnel and Twin Tunnel & 5 & 639,200 & 127,840 \\
\hline U.S. 287 Bypass Toll Road & U.S. 287 to I-25 Connector & 12 & 142,200 & 11,850 \\
\hline Powers Boulevard Toll Road Scenario 1 & I-25N (Northgate Rd.) to Woodmen Rd. & 9 & 175,200 & 19,467 \\
\hline Powers Boulevard Toll Road Scenario 2 & I-25N (Northgate Rd.) to Drennan & 21 & 550,000 & 26,190 \\
\hline Powers Boulevard Toll Road Scenario 3 & I-25N (Northgate Rd.) to New Arterial & 27 & 722,100 & 26,744 \\
\hline Powers Boulevard Toll Road Scenario 4 & Following Drennan Alignment & 12 & 229,600 & 19,133 \\
\hline Banning-Lewis Parkway Toll Road Scenario 1 & I-25N to I-25S at Fountain (unimproved Powers Blvd.) & 31 & 573,600 & 18,503 \\
\hline Banning-Lewis Parkway Toll Road Scenario 2 & I-25N to I-25S at Fountain (improved Powers Blvd.) & 31 & 573,600 & 18,503 \\
\hline Front Range Toll Road Scenario 1 & I-25N (at Fort Collins) to I-25S (south of Pueblo) & 194 & 2,344,100 & 12,083 \\
\hline Front Range Toll Road Scenario 2 & I-25N (at Fort Collins) to I-25S (north of Pueblo) & 169 & 1,979,400 & 11,712 \\
\hline I-25 Sc 1/U.S. 36/I-270/I-70E Sc 3/I-225 Toll System & System & 62 & 2,894,700 & 46,689 \\
\hline
\end{tabular}

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\begin{tabular}{|c|c|c|c|}
\hline \multicolumn{4}{|l|}{\begin{tabular}{l}
Table 5-2 \\
Summary of Toll Collection Capital Cost Estimates Second-Tier Candidate Toll Projects
\end{tabular}} \\
\hline Location & Limits & Length (miles) & Toll Collection Capital Costs (000) \\
\hline I-25 Express Toll Lanes Scenario 1 & U.S. 36 to S.H. 66 & 26 & \$ 7,820 \\
\hline I-25 Express Toll Lanes Scenario 2 & U.S. 36 to S.H. 7 & 12 & 6,640 \\
\hline I-70 East Express Toll Lanes Scenario 1 & I-25 to E-470 & 12 & 4,812 \\
\hline I-70 East Express Toll Lanes Scenario 2 & I-25 to Chambers Road & 8 & 4,577 \\
\hline I-70 East Express Toll Lanes Scenario 3 & Colorado to Chambers Road & 6 & 4,577 \\
\hline U.S. 36 Express Toll Lanes & I-25 to Foothills Parkway & 18 & 7,500 \\
\hline I-225 Express Toll Lanes & I-70 to S.H. 83 & 8 & 3,241 \\
\hline I-270 Express Toll Lanes & I-25 to I-70 & 5 & 3,168 \\
\hline C-470 Express Toll Lanes Scenario 1, 1A & \(\mathrm{I}-25\) to Kipling & 14 & 5,707 \\
\hline C-470 Express Toll Lanes Scenario 2, 2A & I-25 to I-70 & 26 & 7,706 \\
\hline Northwest Corridor Toll Road Scenario 1 & C-470 to NW Parkway/US 36 & 24 & 6,240 \\
\hline Northwest Corridor Toll Road Scenario 2 & SH 128 to SH 58 & 14 & 6,240 \\
\hline I-70 West Toll Road Scenario 1 & West of Eisenhower Tunnel to Floyd Hill & 34 & 7,305 \\
\hline I-70 West Toll Road Scenario 2 & Eisenhower Tunnel to Floyd Hill & 34 & 6,279 \\
\hline I-70 West Toll Road Scenario 3, 3A, 3B & Eisenhower Tunnel and Twin Tunnel & 5 & 6,279 \\
\hline U.S. 287 Bypass Toll Road & U.S. 287 to I-25 Connector & 12 & 2,840 \\
\hline Powers Boulevard Toll Road Scenario 1 & I-25N (Northgate Rd.) to Woodmen Rd. & 9 & 9,022 \\
\hline Powers Boulevard Toll Road Scenario 2 & I-25N (Northgate Rd.) to Drennan & 21 & 13,715 \\
\hline Powers Boulevard Toll Road Scenario 3 & I-25N (Northgate Rd.) to New Arterial & 27 & 16,375 \\
\hline Powers Boulevard Toll Road Scenario 4 & Following Drennan Alignment & 12 & 16,375 \\
\hline Banning-Lewis Parkway Toll Road Scenario 1 & I-25N to I-25S at Fountain (unimproved Powers Blvd.) & 31 & 10,408 \\
\hline Banning-Lewis Parkway Toll Road Scenario 2 & I-25N to I-25S at Fountain (improved Powers Blvd.) & 31 & 10,408 \\
\hline Front Range Toll Road Scenario 1 & I-25N (at Fort Collins) to I-25S (south of Pueblo) & 194 & 17,649 \\
\hline Front Range Toll Road Scenario 2 & I-25N (at Fort Collins) to I-25S (north of Pueblo) & 169 & 16,919 \\
\hline I-25 Sc 1/U.S. 36/I-270/I-70E Sc 3/I-225 Toll System & System & 62 & 23,641 \\
\hline
\end{tabular}

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transactions sent from the lane processing equipment and then forward the ETC transactions to the remote customer service center server.
- Project Delivery - The capital costs for the System Integrator's project management, document preparation in accordance with the contract and successful system and acceptance testing as a condition of acceptance by the Agency.

The primary assumptions made in developing these tables was for single tolling point facilities, a roadside cabinet was used instead of a toll and communication building to reduce trade costs and communication transmission is arranged through the local carrier. Also, host software is licensed from a vendor with no customization and paid for as an operating cost. For multi-tolling point projects, a communication backbone is installed that is routed for the approximate length of the facility to interconnect tolling points and provide flexibility in locating dynamic and changeable signs. Toll and communication buildings are installed at each toll point to provide a more durable and secure structure with a trade cost that can be spread. Facilities that included the toll and communication building also included costs associated with a remotely monitored security access control system. Reversible lane facilities include costs for gate access control. Finally, the tunnel toll plaza project includes manual equipment costs. All capital cost estimates for each project are in 2004 dollars.

\section*{ESTIMATED ROADWAY ANNUAL OPERATIONS AND MAINTENANCE COST}

Annual roadway operations and maintenance (O\&M) costs were developed for each project. The derivation was, in part, based on the experiences of the other turnpike systems currently in operation in Colorado (E-470 and Northwest Parkway), other express toll systems in operation throughout the country, and team experience on other similar toll studies. O\&M costs refer to the perpetual costs associated with the operations and upkeep of the turnpike system. These costs represent the annual revenue necessary to responsibly operate and maintain the toll road in a manner similar with customary practice. The annual roadway O\&M costs for each project included cost estimates for the following cost categories:
- Insurance - The annual costs to insure the toll facility including property, liability and business interruption insurance.

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- Colorado State Patrol (CSP) - The annual costs to employ CSP for the toll facility for enforcement and safety. For the smaller candidate toll projects, this assumed one patrol car operating at all times in three shifts - weekdays, weeknights and weekends. For the larger candidate toll projects, such as the Front Range or the system concepts, a dedicated troop was assumed, operating at all times in three shifts weekdays, weeknights and weekends and includes the vehicle \(\mathrm{O} \& \mathrm{M}\) costs.
- Roadway Maintenance - Roadway maintenance costs are those costs associated with the upkeep of the turnpike pavement and roadside, including snow removal, mowing, sign and guardrail repair, minor bridge repair, and pavement resurfacing. The annual costs to maintain the entire length of the toll facility were developed as an annual cost per lane-mile.
- Facility Maintenance - Facility maintenance is the annual operations and maintenance and utilities for a facility related to roadway and bridge maintenance and equipment, including signing and lighting for the toll facility. The cost is based on the square footage of the facility.
- Engineering/Traffic Consulting - The annual costs associated with retaining an independent engineering and traffic consultant for the toll system.

Table 5-3 shows the roadway annual O\&M cost estimates for each project in 2004 dollars.

\section*{ESTIMATED TOLL COLLECTION ANNUAL OPERATIONS AND MAINTENANCE COST}

Annual toll collection O \& M costs are shown in Table 5-4 for each of the projects. These are the costs incurred annually to operate and maintain the toll system described in detail in Chapters 3 and 4. The two categories of costs that apply to all projects are administration and maintenance. Administration costs are the minimum expenses incurred by the Agency to manage operations and maintenance and perform the audit and reconciliation activities required by statute. This includes daily review of system, revenue and violation performance to identify anomalies and trends. It is assumed these activities will take place at a leased commercial space suited for this operation. The host computer system is assumed to reside in this space and communications to the remote express lane or toll road facility and the regional customer service and violation processing or

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\begin{tabular}{|c|c|c|c|}
\hline \multicolumn{4}{|l|}{\begin{tabular}{l}
Table 5-3 \\
Summary of Roadway Operation and Maintenance Cost Estimates Second-Tier Candidate Toll Projects
\end{tabular}} \\
\hline Location & Limits & Length (miles) & Roadway O\&M Costs \\
\hline I-25 Express Toll Lanes Scenario 1 & U.S. 36 to S.H. 66 & 26 & \$ 1,980,000 \\
\hline I-25 Express Toll Lanes Scenario 2 & U.S. 36 to S.H. 7 & 12 & 1,110,000 \\
\hline I-70 East Express Toll Lanes Scenario 1 & I-25 to E-470 & 12 & 1,370,000 \\
\hline I-70 East Express Toll Lanes Scenario 2 & I-25 to Chambers Road & 8 & 1,100,000 \\
\hline I-70 East Express Toll Lanes Scenario 3 & Colorado to Chambers Road & 6 & 1,000,000 \\
\hline U.S. 36 Express Toll Lanes & I-25 to Foothills Parkway & 18 & 1,690,000 \\
\hline I-225 Express Toll Lanes & I-70 to S.H. 83 & 8 & 1,110,000 \\
\hline I-270 Express Toll Lanes & I-25 to I-70 & 5 & 960,000 \\
\hline C-470 Express Toll Lanes Scenario 1, 1A & I-25 to Kipling & 14 & 1,460,000 \\
\hline C-470 Express Toll Lanes Scenario 2, 2A & I-25 to I-70 & 26 & 2,160,000 \\
\hline Northwest Corridor Toll Road Scenario 1 & C-470 to NW Parkway/US 36 & 24 & 2,040,000 \\
\hline Northwest Corridor Toll Road Scenario 2 & SH 128 to SH 58 & 14 & 1,460,000 \\
\hline I-70 West Toll Road Scenario 1 & West of Eisenhower Tunnel to Floyd Hill & 34 & 2,440,000 \\
\hline I-70 West Toll Road Scenario 2 & Eisenhower Tunnel to Floyd Hill & 34 & 2,440,000 \\
\hline I-70 West Toll Road Scenario 3, 3A, 3B & Eisenhower Tunnel and Twin Tunnel & 5 & 1,360,000 \\
\hline U.S. 287 Bypass Toll Road & U.S. 287 to I-25 Connector & 12 & 1,350,000 \\
\hline Powers Boulevard Toll Road Scenario 1 & I-25N (Northgate Rd.) to Woodmen Rd. & 9 & 1,170,000 \\
\hline Powers Boulevard Toll Road Scenario 2 & I-25N (Northgate Rd.) to Drennan & 21 & 1,870,000 \\
\hline Powers Boulevard Toll Road Scenario 3 & I-25N (Northgate Rd.) to New Arterial & 27 & 2,220,000 \\
\hline Powers Boulevard Toll Road Scenario 4 & Following Drennan Alignment & 12 & 1,370,000 \\
\hline Banning-Lewis Parkway Toll Road Scenario 1 & I-25N to I-25S at Fountain (unimproved Powers Blvd.) & 31 & 2,450,000 \\
\hline Banning-Lewis Parkway Toll Road Scenario 2 & I-25N to I-25S at Fountain (improved Powers Blvd.) & 31 & 2,450,000 \\
\hline Front Range Toll Road Scenario 1 & I-25N (at Fort Collins) to I-25S (south of Pueblo) & 194 & 14,500,000 \\
\hline Front Range Toll Road Scenario 2 & I-25N (at Fort Collins) to I-25S (north of Pueblo) & 169 & 13,050,000 \\
\hline I-25 Sc 1/U.S. 36/I-270/I-70E Sc 3/I-225 Toll System & System & 62 & 6,640,000 \\
\hline
\end{tabular}

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remote service providers is assumed to be interconnected to the local carrier's network using a T1 service. For projects with a single toll point, the vendor field support item includes the cost to license the nondevelopmental, off the shelf host software in addition to maintaining the host hardware including all peripherals.

Projects with multiple toll points do not include host software with this item. For operations, the cost to process, store, transfer, reconcile and report ETC transactions dominates all other operations cost. This cost is derived by using the calculated trips and an industry supported unit price. The only alternative transaction for express lane operations, violation transactions, are assumed to be revenue neutral, meaning all costs incurred to gather and process annual violations is assumed to be equal to the revenue generated from the fees and fines paid by violators receiving a violation citation. Historical performance for a properly designed, implemented, operated and managed violation processing system demonstrating net revenue is generated.

The maintenance category includes the cost to maintain the field level toll system equipment. This work is performed by trained technicians, who must be provided with a vehicle, a cell phone, test equipment, and tools to perform preventative maintenance and restore failures. An inventory of spare parts is also included under this category to make the necessary replacements and repairs. The leased facility is assumed to have space allocated to storing spare parts in addition to what the technician carries in the vehicle. Annual O \& M cost estimates for each project is in 2004 dollars.

\section*{ANNUAL REPLACEMENT FUND DEPOSIT}

Included in the annual costs of a toll system are replacement reserve fund considerations. On an annual basis, the Replacement Fund Deposit needs to be deposited for the replacement of the system's infrastructure to replace or refurbish the system at the end of its service life, assumed to be 30 years. The depreciation of the system's value is a function of the system's use and the extent that annual maintenance activities are able to defer major system reconstruction. It is assumed that upon reaching maturity, the system's driving surface, including the pavement and bridge decks, will require reconstruction in its original configuration. The remaining value of these elements, consisting of the pavement base and the bridge substructure, would depend on the rate of the system's deterioration due to use and weathering. Upgrades of the system for increased capacity demands or new design standards would not be
included. Assuming a typical construction value for these elements of the system's infrastructure of around 15 percent of the original construction costs, necessary deposits into a "sinking" fund are assumed to accrue during the typical life of the system to provide the necessary funds to reconstruct the system upon reaching its service life. Using a discount rate of 6 percent, an annual deposit approximately equaling 0.19 percent of the original construction cost would be necessary during the life of the project. The replacement fund deposits would likely have to be supplemented by potential bond refinancing or sale of additional debt if the costs to reconstruct exceed available monies in this fund. Other considerations such as toll increases and major maintenance bond issues are considerations for additional funds, of course, assuming the project toll revenues could support this process.

For each project, the annual replacement fund deposit value estimated includes only the portion of construction costs and right-of-way associated with the toll facility and was not based on costs associated with improvements/reconstruction of the general purpose lanes.

Table 5-5 shows the annual cost estimate for the Replacement Fund Deposit for each project. In addition, Appendix A shows a summary of the Replacement Fund Deposit for each project.

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\begin{tabular}{|c|c|c|c|}
\hline \multicolumn{4}{|l|}{\begin{tabular}{l}
Table 5-5 \\
Summary of Annual Reserve Maintenance Fund Deposit Cost Estimates Second-Tier Candidate Toll Projects
\end{tabular}} \\
\hline Location & Limits & Length (miles) & Replacement Fund Deposit \\
\hline I-25 Express Toll Lanes Scenario 1 & U.S. 36 to S.H. 66 & 26 & 380,000 \\
\hline I-25 Express Toll Lanes Scenario 2 & U.S. 36 to S.H. 7 & 12 & 290,000 \\
\hline I-70 East Express Toll Lanes Scenario 1 & I-25 to E-470 & 12 & 660,000 \\
\hline I-70 East Express Toll Lanes Scenario 2 & I-25 to Chambers Road & 8 & 570,000 \\
\hline I-70 East Express Toll Lanes Scenario 3 & Colorado to Chambers Road & 6 & 260,000 \\
\hline U.S. 36 Express Toll Lanes & I-25 to Foothills Parkway & 18 & 1,150,000 \\
\hline I-225 Express Toll Lanes & I-70 to S.H. 83 & 8 & 200,000 \\
\hline I-270 Express Toll Lanes & I-25 to I-70 & 5 & 200,000 \\
\hline C-470 Express Toll Lanes Scenario 1, 1A & I-25 to Kipling & 14 & 540,000 \\
\hline C-470 Express Toll Lanes Scenario 2, 2A & I-25 to I-70 & 26 & 880,000 \\
\hline Northwest Corridor Toll Road Scenario 1 & C-470 to NW Parkway/US 36 & 24 & 1,390,000 \\
\hline Northwest Corridor Toll Road Scenario 2 & SH 128 to SH 58 & 14 & 550,000 \\
\hline I-70 West Toll Road Scenario 1 & West of Eisenhower Tunnel to Floyd Hill & 34 & 3,310,000 \\
\hline I-70 West Toll Road Scenario 2 & Eisenhower Tunnel to Floyd Hill & 34 & 3,160,000 \\
\hline I-70 West Toll Road Scenario 3, 3A, 3B & Eisenhower Tunnel and Twin Tunnel & 5 & 1,080,000 \\
\hline U.S. 287 Bypass Toll Road & U.S. 287 to I-25 Connector & 12 & 240,000 \\
\hline Powers Boulevard Toll Road Scenario 1 & I-25N (Northgate Rd.) to Woodmen Rd. & 9 & 150,000 \\
\hline Powers Boulevard Toll Road Scenario 2 & I-25N (Northgate Rd.) to Drennan & 21 & 470,000 \\
\hline Powers Boulevard Toll Road Scenario 3 & I-25N (Northgate Rd.) to New Arterial & 27 & 620,000 \\
\hline Powers Boulevard Toll Road Scenario 4 & Following Drennan Alignment & 12 & 200,000 \\
\hline Banning-Lewis Parkway Toll Road Scenario 1 & I-25N to I-25S at Fountain (unimproved Powers Blvd.) & 31 & 980,000 \\
\hline Banning-Lewis Parkway Toll Road Scenario 2 & I-25N to I-25S at Fountain (improved Powers Blvd.) & 31 & 980,000 \\
\hline Front Range Toll Road Scenario 1 & I-25N (at Fort Collins) to I-25S (south of Pueblo) & 194 & 4,040,000 \\
\hline Front Range Toll Road Scenario 2 & I-25N (at Fort Collins) to I-25S (north of Pueblo) & 169 & 3,400,000 \\
\hline I-25 Sc 1/U.S. 36/I-270/I-70E Sc 3/I-225 Toll System & System & 62 & 2,750,000 \\
\hline
\end{tabular}


The study team, including participants from Citigroup, WSA and HNTB, evaluated the financial feasibility of the CDOT's second-tier candidate toll projects to assist CDOT in determining the priority and economic feasibility of the projects. This comprehensive evaluation encompassed 12 individual express toll and/or managed lane projects, including multiple construction/design approaches for certain projects. In all, the financial feasibility for 28 individual project scenarios was reviewed.

Three main themes resulted from this analysis:
1) Targeting for early completion programs that can fully fund construction costs through toll revenues (i.e., without requiring federal, state and/or local monies);
2) Combining certain toll roads into a "Regional System" allows the more economical toll roads to "leverage up" less economical toll roads, resulting in a more efficient use of toll revenues, reduced total dependence on governmental monies, and provides for a more cohesive financing; and
3) Supporting projects with some federal/state monies to enhance statewide project completion feasibility.

\section*{METHODOLOGY FOR PRO FORMAS}

The study team evaluated the 28 second-tier toll projects based on a variety of assumptions provided by WSA and HNTB, and the financial feasibility analyses performed by Citigroup. The assumptions incorporated into the analyses include project capital costs, annual toll revenues, operations and maintenance costs (both roadway and toll collection), and renewal and replacement fund deposits. Each project assumed an opening date of January 1, 2010 and a three-year construction period.

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The project cost factors share the following characteristics:
- Project Costs - provided in 2004 dollars, inflated at 5.0 percent annually to 2010;
- Annual Toll Revenues - provided in 2004 dollars, inflated at 2.5 percent from 2004 to year of revenue generation;
- Roadway and Toll Collection Operations and Maintenance - provided in 2004 dollars, inflated at 3.0 percent from 2004 to year of incurred expense; and
- Annual Renewal and Replacement Fund Deposit - provided in 2004 dollars, inflated at 3.0 percent from 2004 to year of incurred expense.

Each project was evaluated utilizing the same financial methodology:
First, the total costs for each scenario assumed the combination of project costs and bond costs. Bond costs for each scenario incorporated the following assumptions:
- Cost of Issuance - assumed at 2.0 percent of total senior lien bonds to fund estimated standard bond issuance expenses including legal fees, underwriting fees and rating agency fees, among others;
- Capitalized Interest - three years;
- Interest Earnings on Capitalized Interest and Construction Fund - 1.5 percent for three years;
- Construction Fund Adjustment - 4.5 percent loss on fund balance for three years (difference between borrowing cost and fund earnings);
- Debt Service Reserve Fund - 10.0 percent of senior lien principal; and
- Interest Rates - Current rates and, for the market sensitivity analysis, current rates plus 100 bps.

Second, each scenario was stressed to maximize the amount of senior lien bonds that could be issued, subject to certain constraints. These constraints, as listed below, are those likely to be imposed upon a start-up toll road bond program by rating agencies, bond insurers and/or investors.
- Principal Amortization Period - 30-years;
- Senior Lien Coverage Requirement - 1.75 times net revenues. Net revenues equal gross toll revenues less annual operation and maintenance expenses, plus annual debt service reserve fund interest earnings;
- Interest Rates on Senior Lien Current Interest Bonds - rates of August 9, 2004; and
- Interest Rates on Senior Lien Capital Appreciation Bonds - Current interest bond rates of August 9, 2004 plus 0.75 percent.


The financial methodology employed is based on industry practice and comparable startup toll road methodologies. Startup toll roads’ senior lien financial structure must be rated at least "investment grade" ("BBB-" or greater) by one of the three major rating agencies to obtain efficient, broad market access. In general, ratings agencies assign BBB- credit ratings to start-up toll roads that meet a minimum senior lien coverage constraint of 1.75 times, have a reliable traffic and revenue study and have a strong management team. This credit assessment is especially true for toll facilities when not all lanes are tolled and when revenues must be generated in a concentrated time period. The coverage for a toll road is calculated by dividing total net revenues by total debt service (i.e., the road must project at least \(\$ 1.75\) in annual net revenues for each \(\$ 1.00\) of annual bond debt service).

Table 6-1 lists each scenario in order of financial feasibility. As later discussed in the section, "Evaluation of Denver Regional Area Projects," the rows titled "Denver Area Projects—Scenario 1" and "Denver Area Projects—Scenario 2" comprise toll systems of all Denver Regional Area Projects. These two Scenarios differ in their assumptions regarding the inclusion of federal and state/local transfers, also as later discussed.

\section*{REVIEW OF ALTERNATIVES EVALUATED}

In Table 6-1, the alternatives are presented in order of "Percentage of Project Cost," representing the percentage of each project's costs paid from a maximum issuance of senior lien bonds (subject to the previously mentioned constraints) and equity contributions from federal, state and/or local sources (also subject to constraints, as described in the next paragraph). Projects above the blackline are those able to fund at least 70 percent of total project costs through these sources, and thus are deemed more probably financially feasible. Upon review of the projects and comparable industry standards, the study team concluded that such projects have a strong likelihood of financial feasibility as either additional senior bonds or subordinated bonds (with slightly lower coverage constraints of 1.30 times combined debt service coverage) could fund the remaining project costs.


Table 6-1
Summary of All Alternatives Evaluated
\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline \multirow[b]{2}{*}{Project} & \multirow[t]{2}{*}{Total 2010 Project Cost with COI} & \multirow[t]{2}{*}{Senior Lien Proceeds Par Amount} & \multicolumn{2}{|l|}{Federal \({ }^{(1)}\)} & \multirow[b]{2}{*}{Shortfall/ (Excess)} & \multirow[b]{2}{*}{\% of Project
Cost} & \multicolumn{3}{|c|}{State \& Local} \\
\hline & & & Upfront Transfers \% & Transfers \$ & & & Annual
Transfers \% & \[
\begin{gathered}
\text { Gross } \\
\text { Transfers \$ }{ }^{(2)} \\
\hline
\end{gathered}
\] & \[
\begin{gathered}
\text { Net } \\
\text { Transfers \$ }{ }^{(3)} \\
\hline
\end{gathered}
\] \\
\hline 1-70E Express Toll Lanes Scenario 3 & 293,799,057 & 293,803,307 & 0.00\% & & \((4,250)\) & 100.00\% & 0.00\% & - & - \\
\hline 1-70 Mountain Corridor - Scenario 3 & 1,097,606,741 & 1,097,609,009 & 0.00\% & - & \((2,268)\) & 100.00\% & 0.00\% & - & - \\
\hline 1-25 Express Toll Lanes Scenario 1 & 522,092,110 & 522,092,920 & 0.00\% & & (809) & 100.00\% & 0.00\% & & \\
\hline 1-70 Mountain Corridor - Scenario 3A & 1,071,884,739 & 883,258,993 & 0.00\% & & 188,625,746 & 82.40\% & 0.00\% & - & \\
\hline 1-225 Express Toll Lanes & 290,149,773 & 237,603,245 & 0.00\% & - & 52,546,529 & 81.89\% & 0.00\% & & - \\
\hline Powers Toll Road Scenario 2 & 933,255,559 & 747,768,444 & 0.00\% & - & 185,487,115 & 80.12\% & 0.00\% & - & \\
\hline Powers Toll Road Scenario 3 & 1,210,713,055 & 879,441,589 & 0.00\% & - & 331,271,467 & 72.64\% & 0.00\% & & \\
\hline 1-270 Express Toll Lanes & 342,000,226 & 244,726,949 & 0.00\% & - & 97,273,277 & 71.56\% & 0.00\% & - & \\
\hline C-470 Express Toll Lanes Scenario 1A & 522,559,134 & 364,844,370 & 0.22\% & 943,015 & 156,771,749 & 70.00\% & 0.00\% & - & - \\
\hline C-470 Express Toll Lanes Scenario 2A & 852,240,365 & 578,498,911 & 2.58\% & 18,036,151 & 255,705,303 & 70.00\% & 0.00\% & & \\
\hline Powers Toll Road Scenario 4 & 394,169,608 & 243,542,868 & 10.01\% & 32,365,222 & 118,261,518 & 70.00\% & 0.00\% & - & \\
\hline 1-25 Express Toll Lanes Scenario 2 & 379,744,624 & 219,101,717 & 15.00\% & 46,724,218 & 113,918,689 & 70.00\% & 0.00\% & - & - \\
\hline Northwest Corridor Scenario 2 & 526,511,749 & 297,186,533 & 16.52\% & 71,347,792 & 157,977,424 & 70.00\% & 0.00\% & - & \\
\hline 1-70 Mountain Corridor - Scenario 3B & 1,054,471,523 & 593,874,304 & 16.68\% & 144,274,552 & 316,322,667 & 70.00\% & 0.00\% & - & - \\
\hline Powers Toll Road Scenario 1 & 300,882,982 & 168,314,504 & 17.14\% & 42,302,735 & 90,265,743 & 70.00\% & 0.00\% & - & \\
\hline Denver Area Projects Scenario 2 & 4,772,150,614 & 2,581,988,481 & 19.38\% & 758,623,843 & 1,431,538,290 & 70.00\% & 0.00\% & - & - \\
\hline 1-70E Express Toll Lanes Scenario 2 & 822,058,672 & 431,498,113 & 20.00\% & 135,053,992 & 255,506,567 & 68.92\% & 10.00\% & 229,749,539 & 89,719,374 \\
\hline C-470 Express Toll Lanes Scenario 1 & 519,767,857 & 256,798,145 & 20.00\% & 85,728,598 & 177,241,113 & 65.90\% & 10.00\% & 137,088,844 & 56,055,554 \\
\hline I-70E Express Toll Lanes Scenario 1 & 942,486,280 & 465,372,174 & 20.00\% & 155,456,018 & 321,658,088 & 65.87\% & 10.00\% & 250,937,393 & 97,240,283 \\
\hline C-470 Express Toll Lanes Scenario 2 & 845,303,390 & 398,911,804 & 20.00\% & 139,815,126 & 306,576,459 & 63.73\% & 10.00\% & 208,463,594 & 85,359,182 \\
\hline Denver Area Projects Scenario 1 & 4,728,017,529 & 2,558,193,766 & 10.90\% & 414,642,845 & 1,755,180,917 & 62.88\% & 1.41\% & 223,153,273 & 85,521,879 \\
\hline Northwest Corridor Scenario 1 & 1,383,715,935 & 590,950,360 & 20.00\% & 230,175,248 & 562,590,327 & 59.34\% & 10.00\% & 286,314,081 & 120,009,645 \\
\hline Front Range Toll Road Express Toll Lanes Scenario 2 & 3,206,570,456 & 1,291,301,470 & 20.00\% & 535,043,465 & 1,380,224,485 & 56.96\% & 10.00\% & 645,201,105 & 271,532,386 \\
\hline 1-70 Mountain Corridor - Scenario 2 & 3,973,487,116 & 1,436,659,073 & 20.00\% & 666,449,664 & 1,870,378,380 & 52.93\% & 10.00\% & 610,475,250 & 268,215,850 \\
\hline Front Range Toll Road Express Toll Lanes Scenario 1 & 3,768,725,282 & 1,321,022,578 & 20.00\% & 632,981,471 & 1,814,721,233 & 51.85\% & 10.00\% & 675,303,371 & 284,841,439 \\
\hline Banning-Lewis Parkway Toll Road - Scenario 2 & 918,411,929 & 214,320,663 & 20.00\% & 156,517,093 & 547,574,173 & 40.38\% & 10.00\% & 134,309,112 & 54,563,285 \\
\hline Banning-Lewis Parkway Tollroad - Scenario 1 & 917,972,752 & 210,660,853 & 20.00\% & 156,517,093 & 550,794,806 & 40.00\% & 10.00\% & 138,215,349 & 54,660,250 \\
\hline U.S. 36 Express Toll Lanes & 1,901,224,249 & 384,281,903 & 20.00\% & 325,258,901 & 1,191,683,445 & 37.32\% & 10.00\% & 223,153,273 & 85,521,879 \\
\hline 1-70 Mountain Corridor - Scenario 1 & 4,025,922,619 & 291,138,085 & 20.00\% & 699,744,728 & 3,035,039,805 & 24.61\% & 10.00\% & 225,471,439 & 76,317,237 \\
\hline U.S. 287 Corridor Express Toll Lanes & 222,180,746 & 4,270,319 & 20.00\% & 38,865,390 & 179,045,037 & 19.41\% & 10.00\% & 15,587,417 & 6,651,248 \\
\hline
\end{tabular}

\footnotetext{
= Denver Regional Area Projects Selected for Cashflow
\({ }^{(1)}\) Upfront transfers include federal moneys available in the form of a one-time, upfront payment
\({ }^{(2)}\) Gross transfers include the total annual state and local contributions over the life of the program
\({ }^{(3)}\) Net transfers are the present value at \(5.00 \%\) of the gross transfers to the year 2010
}

The additional federal, state and/or local equity contributions mentioned above were provided to the extent that senior lien bonds from leveraged toll revenues could not fund at least 70 percent of total project costs, subject to certain limitations. First, federal monies could be available in the form of a one-time, upfront payment. This upfront payment is limited to 20 percent of total 2010 project capital costs (exclusive of bond costs). Second, state and local contributions could be available as an annual transfer of up to 10 percent of total gross toll revenues generated for a specific project in a respective year. This annual transfer amount meets all TABOR restrictions. The need for transfers was determined in a four-step process:
1) Maximize senior lien bonds to meet at least 70 percent of total project costs;
2) If less than 70 percent of total project costs remain unfunded, utilize federal upfront contribution of up to 20 percent of 2010 project costs;
3) If less than 70 percent of total project costs remain unfunded after senior lien bond issuance and upfront federal contribution, utilize

annual state and local transfers up to 10 percent of gross toll revenues for a respective year; and
4) If less than 70 percent of total project costs remain unfunded after senior lien bond issuance, upfront federal contribution, and state/local annual transfers, project is deemed infeasible and falls below the blackline.

\section*{SELECTION OF ALTERNATIVES}

Upon review of Table 6-1, for the individual projects containing multiple possible scenarios, the study team identified one scenario as the "Selected Alternative" for each project based on maximizing financial feasibility. However, the study team continues to present both Scenarios 1 and 2 for the Denver Area Projects as these two scenarios have different financing assumptions. Table 6-2, following, shows those projects selected by the study team, and with CDOT's review and concurrence, as the Selected Alternatives.

Table 6-2
Summary of Selected Alternatives
\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline \multirow[b]{2}{*}{Project} & \multirow[t]{2}{*}{Total 2010 Project Cost with COI} & \multirow[t]{2}{*}{Senior Lien Proceeds Par Amount} & \multicolumn{2}{|c|}{Federal \({ }^{(1)}\)} & \multirow[b]{2}{*}{Shortfall/ (Excess)} & \multirow[b]{2}{*}{\% of Project
Cost} & \multicolumn{3}{|c|}{State \& Local} \\
\hline & & & Upfront
Transfers \% & Transfers \$ & & & Annual
Transfers \% & \[
\begin{gathered}
\text { Gross } \\
\text { Transfers \$ }{ }^{(k)}
\end{gathered}
\] & \[
\begin{gathered}
\text { Net } \\
\text { Transfers } \$^{(3)}
\end{gathered}
\] \\
\hline 1-70E Express Toll Lanes Scenario 3 & 293,799,057 & 293,803,307 & 0.00\% & & \((4,250)\) & 100.00\% & 0.00\% & & \\
\hline 1-70 Mountain Corridor - Scenario 3 & 1,097,606,741 & 1,097,609,009 & 0.00\% & & \((2,268)\) & 100.00\% & 0.00\% & & \\
\hline I-25 Express Toll Lanes Scenario 1 & 522,092,110 & 522,092,920 & 0.00\% & & (809) & 100.00\% & 0.00\% & & - \\
\hline I-225 Express Toll Lanes & 290,149,773 & 237,603,245 & 0.00\% & & 52,546,529 & 81.89\% & 0.00\% & & \\
\hline 1-270 Express Toll Lanes & 342,000,226 & 244,726,949 & 0.00\% & - & 97,273,277 & 71.56\% & 0.00\% & & \\
\hline C-470 Express Toll Lanes Scenario 2A & 852,240,365 & 578,498,911 & 2.58\% & 18,036,151 & 255,705,303 & 70.00\% & 0.00\% & - & \\
\hline Powers Toll Road Scenario 4 & 394,169,608 & 243,542,868 & 10.01\% & 32,365,222 & 118,261,518 & 70.00\% & 0.00\% & - & - \\
\hline Northwest Corridor Scenario 2 & 526,511,749 & 297,186,533 & 16.52\% & 71,347,792 & 157,977,424 & 70.00\% & 0.00\% & - & \\
\hline Denver Area Projects Scenario 2 & 4,772,150,614 & 2,581,988,481 & 19.38\% & 758,623,843 & 1,431,538,290 & 70.00\% & 0.00\% & - & \\
\hline Denver Area Projects Scenario 1 & 4,728,017,529 & 2,558,193,766 & 10.90\% & 414,642,845 & 1,755,180,917 & 62.88\% & 1.41\% & 223,153,273 & 85,521,879 \\
\hline Front Range Toll Road Express Toll Lanes Scenario 1 & 3,768,725,282 & 1,321,022,578 & 20.00\% & 632,981,471 & 1,814,721,233 & 51.85\% & 10.00\% & 675,303,371 & 284,841,439 \\
\hline Banning-Lewis Parkway Toll Road - Scenario 2 & 918,411,929 & 214,320,663 & 20.00\% & 156,517,093 & 547,574,173 & 40.38\% & 10.00\% & 134,309,112 & 54,563,285 \\
\hline U.S. 36 Express Toll Lanes & 1,901,224,249 & 384,281,903 & 20.00\% & 325,258,901 & 1,191,683,445 & 37.32\% & 10.00\% & 223,153,273 & 85,521,879 \\
\hline U.S. 287 Corridor Express Toll Lanes & 222,180,746 & 4,270,319 & 20.00\% & 38,865,390 & 179,045,037 & 19.41\% & 10.00\% & 15,587,417 & 6,651,248 \\
\hline
\end{tabular}
= Denver Regional Area Projects Selected for Cashflow
\({ }^{(1)}\) Upfront transfers include federal moneys available in the form of a one-time, upfront payment
\({ }^{(2)}\) Gross transfers include the total annual state and local contributions over the life of the program
\({ }^{(3)}\) Net transfers are the present value at \(5.00 \%\) of the gross transfers to the year 2010

As represented in Table 6-2, all projects in the Denver Regional Area, except for U.S. 36 Express Toll Lanes, are "financially feasible" on a stand-alone basis when using the 70 percent threshold (under the assumption that additional senior or subordinated debt would fund the remaining 30 percent of project costs).


Each Denver Area Project labeled Scenario 1 and Scenario 2 is a "System Credit," containing all projects in the Denver Area (with the individual projects shaded in grey). The system credit combines revenues and costs from each individual project, thus allowing surplus revenues from the most financially feasible toll roads to benefit other projects in the system credit. It should be noted that the Fast Tracks contribution for BRT has not been included in the financing assumptions.

Scenarios 1 and 2 differ in that in Scenario 1 calculates the maximum federal and state/local subsidy on a project-by-project basis, whereas Scenario 2 calculates the maximum federal and state/local subsidy on an aggregate project basis (combining all costs and forecasted revenues as one project).

Although the Denver Regional Area Projects Scenario 2 is financially feasible with the maximum amount of federal monies allocated, and some state/local annual transfers, the bulk of all Denver Regional Area Projects’ surplus revenue and federal and state/local subsidies are paying for the U.S. 36 project. It is important to note that this "system credit" approach (Scenario 2) will only be feasible to the extent a strong prioritization of projects exists and the overall system credit has a strong financial history before the U.S. 36 project is financed. Since the U.S. 36 project requires a higher allocation of federal and state/local subsidies, and because it draws upon other projects’ surplus revenue, it is only feasible to complete this project after those projects that stand on their own.

\section*{FEASIBILITY SUMMARY}

Based on the results for the Denver Regional Area Projects, approximately \(\$ 4.7\) billion of project costs can be financed (including costs of issuance) with \(\$ 414\) million in equity contributions from CDOT under Scenario 1 and with \(\$ 759\) million in equity contributions under Scenario 2. This means with at least a 13 or 20 percent upfront contribution to projects for Scenarios 1 and 2, respectively, CDOT may complete major corridor improvements. Supplementing the benefits of CDOT's equity contributions is the option that as a system credit, monies transferred to projects can be paid back to CDOT over time. The study team expects that annual transfers can be paid back first, then any monies in excess after all payments will be available to repay CDOT for any upfront federal contributions to the system costs. Tables 6-3 and 6-4, represent the potential repayment to CDOT from CTE for contributions.

The column labeled "Total Excess Revenues to Repay Transfers" in each table reflects the monies available annually to repay CDOT for any state and local transfers made. The following column, "Total Transfers Estimate" represents the amount of state and local monies transferred from CDOT, and the last column, "Excess after State and Local Transfer Repaid," reflects the amount of monies in the CTE excess fund after debt service, operations and maintenance are paid and CDOT state and local transfers are repaid. This excess monies column is shown on a cumulative basis and may be used to repay CDOT for any upfront Federal monies provided to a project or, at guidance by CDOT, to other capital construction projects.

It should be noted that in Table 6-3, while Denver Regional Area Projects Scenario 1 appears to provide greater surplus, it does not fully fund U.S. 36 to the 70 percent threshold.

Table 6-3
Denver Regional Area Projects-Scenario 1 Cashflow of Funds to Repay Transfers
\(\left.\begin{array}{rrrrrrrr} \\ \text { Date } & \begin{array}{c}\text { Total Non-System } \\ \text { Revenue }\end{array} & \begin{array}{c}\text { Total Non-System } \\ \text { O\&M }\end{array} & \begin{array}{c}\text { Estimated DSRF } \\ \text { Earnings }\end{array} & \begin{array}{c}\text { Total Senior } \\ \text { Lien DS }\end{array} & \begin{array}{c}\text { Estimated } \\ \text { Sub DS * }\end{array} & \begin{array}{c}\text { Total Excess } \\ \text { Revenues to } \\ \text { Repay Transfers }\end{array} & \begin{array}{c}\text { Total Transfers } \\ \text { Estimate }\end{array} \\ \hline & & & & & \\ \text { Excess After S\&L } \\ \text { Transfer Repaid }\end{array}\right]\)
* Subdebt estimated at \(1.30 x\) coverage constraint. Senior Lien at \(1.75 x\) coverage constraint, per model sheet shown

Table 6-4
Denver Area Projects-Scenario 2 Cashflow of Funds to Repay Transfers
\begin{tabular}{|c|c|c|c|c|c|c|c|c|}
\hline Date & Total Non-System
\(\qquad\) & \[
\begin{gathered}
\text { Total Non-System } \\
\text { O\&M }
\end{gathered}
\] & Estimated DSRF
Earnings & Total Senior
Lien DS & Estimated Sub DS * & Total Excess Revenues to Repay Transfers & Total Transfers Estimate & Excess After S\&L Transfer Repaid \\
\hline 6/1/10 & 131,745,811 & 36,564,269 & 12,909,942 & 61,767,052 & 23,756,558 & 22,567,874 & - & 22,567,874 \\
\hline 6/1/11 & 142,740,689 & 37,909,885 & 12,909,942 & 67,278,720 & 25,876,431 & 24,585,597 & & 47,153,470 \\
\hline 6/1/12 & 154,741,819 & 39,307,236 & 12,909,942 & 73,337,870 & 28,206,873 & 26,799,783 & & 73,953,253 \\
\hline 6/1/13 & 167,846,768 & 40,758,401 & 12,909,942 & 80,000,462 & 30,769,408 & 29,228,439 & & 103,181,692 \\
\hline 6/1/14 & 182,162,948 & 42,265,546 & 12,909,942 & 87,316,032 & 33,583,089 & 31,908,224 & & 135,089,916 \\
\hline 6/1/15 & 197,808,640 & 43,830,926 & 12,909,942 & 95,364,160 & 36,678,523 & 34,844,973 & & 169,934,889 \\
\hline 6/1/16 & 214,914,125 & 45,456,894 & 12,909,942 & 104,210,182 & 40,080,839 & 38,076,153 & & 208,011,041 \\
\hline 6/1/17 & 233,622,942 & 47,145,897 & 12,909,942 & 113,937,902 & 43,822,270 & 41,626,815 & & 249,637,857 \\
\hline 6/1/18 & 254,093,280 & 48,900,491 & 12,909,942 & 124,631,931 & 47,935,358 & 45,535,442 & & 295,173,299 \\
\hline 6/1/19 & 276,499,513 & 50,723,334 & 12,909,942 & 136,389,696 & 52,457,575 & 49,838,850 & & 345,012,150 \\
\hline 6/1/20 & 301,033,914 & 52,617,202 & 12,909,942 & 149,329,498 & 57,434,422 & 54,562,734 & & 399,574,884 \\
\hline 6/1/21 & 327,908,540 & 54,584,983 & 12,909,942 & 163,560,887 & 62,908,033 & 59,764,578 & & 459,339,463 \\
\hline 6/1/22 & 357,357,329 & 56,629,691 & 12,909,942 & 179,219,515 & 68,930,583 & 65,487,483 & & 524,826,945 \\
\hline 6/1/23 & 389,638,429 & 58,754,467 & 12,909,942 & 196,453,076 & 75,558,875 & 71,781,953 & & 596,608,899 \\
\hline 6/1/24 & 425,036,768 & 60,962,584 & 12,909,942 & 215,417,800 & 82,853,000 & 78,713,326 & & 675,322,225 \\
\hline 6/1/25 & 463,866,916 & 63,257,457 & 12,909,942 & 236,297,332 & 90,883,589 & 86,338,480 & & 761,660,705 \\
\hline 6/1/26 & 495,089,237 & 65,642,643 & 12,909,942 & 252,776,678 & 97,221,799 & 92,358,060 & & 854,018,764 \\
\hline 6/1/27 & 528,488,523 & 68,121,854 & 12,909,942 & 270,443,249 & 104,016,634 & 98,816,729 & - & 952,835,493 \\
\hline 6/1/28 & 564,221,022 & 70,698,958 & 12,909,942 & 289,387,961 & 111,303,062 & 105,740,984 & - & 1,058,576,477 \\
\hline 6/1/29 & 602,454,480 & 73,377,991 & 12,909,942 & 309,704,477 & 119,117,106 & 113,164,848 & - & 1,171,741,326 \\
\hline 6/1/30 & 643,369,008 & 76,163,160 & 12,909,942 & 331,495,704 & 127,498,348 & 121,121,738 & - & 1,292,863,064 \\
\hline 6/1/31 & 680,311,089 & 79,058,854 & 12,909,942 & 350,950,854 & 134,981,098 & 128,230,225 & - & 1,421,093,289 \\
\hline 6/1/32 & 720,138,367 & 82,069,648 & 12,909,942 & 371,990,124 & 143,073,125 & 135,915,413 & - & 1,557,008,702 \\
\hline 6/1/33 & 762,383,921 & 85,200,317 & 12,909,942 & 394,338,712 & 151,668,735 & 144,086,099 & - & 1,701,094,802 \\
\hline 6/1/34 & 807,199,528 & 88,455,840 & 12,909,942 & 418,086,249 & 160,802,403 & 152,764,977 & - & 1,853,859,779 \\
\hline 6/1/35 & 854,746,786 & 91,841,411 & 12,909,942 & 443,322,379 & 170,508,607 & 161,984,332 & - & 2,015,844,111 \\
\hline 6/1/36 & 897,179,496 & 95,362,447 & 12,909,942 & 465,560,699 & 179,061,807 & 170,104,485 & - & 2,185,948,596 \\
\hline 6/1/37 & 941,799,493 & 99,024,603 & 12,909,942 & 488,964,829 & 188,063,396 & 178,656,608 & - & 2,364,605,204 \\
\hline 6/1/38 & 988,724,038 & 102,833,775 & 12,909,942 & 513,597,809 & 197,537,619 & 187,664,777 & - & 2,552,269,981 \\
\hline 6/1/39 & 1,038,076,928 & 106,796,117 & 12,909,942 & 539,537,099 & 207,514,269 & 197,139,386 & - & 2,749,409,367 \\
\hline 6/1/40 & 1,089,988,872 & 110,918,049 & 12,909,942 & 566,846,273 & 218,017,797 & 207,116,695 & - & 2,956,526,062 \\
\hline Total & 15,835,189,220 & 2,075,234,932 & 400,208,215 & 8,091,515,208 & 3,112,121,234 & 2,956,526,062 & & \\
\hline
\end{tabular}
* Subdebt estimated at \(1.30 x\) coverage constraint. Senior Lien at \(1.75 x\) coverage constraint, per model sheet shown

\section*{MARKET SENSITIVITY ANALYSIS}

As interest rates fall, each project's ability to leverage debt increases thereby increasing its feasibility. Conversely, as interest rates rise, each project's ability to leverage debt decreases, which then lowers its feasibility. Clearly, the current interest rate environment affects the overall feasibility of each project. In order to represent the effect of market movements on these analyses, each project was evaluated reflecting an increase in market rates by an addition of 100 basis points (1.0 percent) to current market rates. The tables for "All Scenarios," Tables 6-5 and 6-6, are as follows.
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Table 6-5

\section*{Costs Associated with Entire Construction - All Alternatives, Market +100 bps}
\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline \multirow[b]{2}{*}{Project} & Total 2010 & Senior Lien & \multicolumn{2}{|c|}{Federal \({ }^{(1)}\)} & \multirow[b]{2}{*}{Shortfall/ (Excess)} & \multirow[b]{2}{*}{\% of Project Cost} & \multicolumn{3}{|c|}{State \& Local} \\
\hline & \[
\begin{aligned}
& \text { Project Cost } \\
& \text { with COI } \\
& \hline
\end{aligned}
\] & \begin{tabular}{l}
Proceeds \\
Par Amount
\end{tabular} & \[
\begin{gathered}
\text { Upfront } \\
\text { Transfers \% } \\
\hline
\end{gathered}
\] & Transfers \$ & & & Annual
Transfers \% & \[
\begin{gathered}
\text { Gross } \\
\text { Transfers \$ }{ }^{(2)}
\end{gathered}
\] & \[
\begin{gathered}
\text { Net } \\
\text { Transfers \$ }{ }^{(3)}
\end{gathered}
\] \\
\hline 1-70E Express Toll Lanes Scenario 3 & 293,798,477 & 293,798,481 & 0.00\% & - & (4) & 100.00\% & 0.00\% & - & \\
\hline I-70 Mountain Corridor - Scenario 3 & 1,097,605,975 & 1,097,602,625 & 0.00\% & & 3,350 & 100.00\% & 0.00\% & - & \\
\hline 1-25 Express Toll Lanes Scenario 1 & 512,469,481 & 441,904,339 & 0.00\% & & 70,565,141 & 86.23\% & 0.00\% & & \\
\hline 1-70 Mountain Corridor - Scenario 3A & 1,058,504,240 & 771,754,834 & 0.00\% & & 286,749,406 & 72.91\% & 0.00\% & & \\
\hline 1-225 Express Toll Lanes & 285,947,794 & 202,586,754 & 0.00\% & & 83,361,040 & 70.85\% & 0.00\% & & \\
\hline Powers Toll Road Scenario 2 & 920,876,029 & 644,605,693 & 0.00\% & & 276,270,336 & 70.00\% & 0.00\% & & \\
\hline Powers Toll Road Scenario 3 & 1,206,528,019 & 757,572,888 & 8.79\% & 86,993,397 & 361,961,733 & 70.00\% & 0.00\% & & \\
\hline 1-270 Express Toll Lanes & 341,301,223 & 208,666,022 & 10.80\% & 30,235,903 & 102,399,298 & 70.00\% & 0.00\% & & - \\
\hline C-470 Express Toll Lanes Scenario 1A & 522,562,852 & 308,251,615 & 13.43\% & 57,566,754 & 156,744,483 & 70.00\% & 0.00\% & - & - \\
\hline C-470 Express Toll Lanes Scenario 2A & 852,245,201 & 490,245,955 & 15.21\% & 106,329,404 & 255,669,843 & 70.00\% & 0.00\% & - & - \\
\hline Powers Toll Road Scenario 4 & 394,171,598 & 211,258,891 & 20.00\% & 64,665,779 & 118,246,928 & 70.00\% & 0.88\% & 13,020,086 & 5,363,641 \\
\hline Northwest Corridor Scenario 2 & 526,511,457 & 282,154,424 & 20.00\% & 86,377,473 & 157,979,560 & 70.00\% & 7.00\% & 115,029,237 & 50,172,859 \\
\hline 1-70 Mountain Corridor - Scenario 3B & 1,054,474,843 & 565,185,459 & 20.00\% & 172,991,070 & 316,298,314 & 70.00\% & 7.78\% & 228,694,667 & 100,289,176 \\
\hline 1-25 Express Toll Lanes Scenario 2 & 379,741,615 & 203,501,902 & 20.00\% & 62,298,957 & 113,940,756 & 70.00\% & 7.90\% & 104,363,248 & 40,789,729 \\
\hline Powers Toll Road Scenario 1 & 300,885,041 & 161,272,976 & 20.00\% & 49,361,418 & 90,250,647 & 70.00\% & 8.33\% & 87,532,702 & 36,128,017 \\
\hline Denver Area Projects Scenario 2 & 4,760,118,716 & 2,457,452,967 & 20.00\% & 782,893,543 & 1,519,772,207 & 68.07\% & 10.00\% & 1,583,518,922 & 612,348,278 \\
\hline I-70E Express Toll Lanes Scenario 2 & 814,199,593 & 366,005,788 & 20.00\% & 135,053,992 & 313,139,812 & 61.54\% & 10.00\% & 229,749,539 & 89,719,374 \\
\hline C-470 Express Toll Lanes Scenario 1 & 515,484,201 & 221,101,017 & 20.00\% & 85,728,598 & 208,654,586 & 59.52\% & 10.00\% & 137,088,844 & 56,055,554 \\
\hline Denver Area Projects Scenario 1 & 4,706,535,729 & 2,245,619,933 & 14.41\% & 548,201,680 & 1,912,714,116 & 59.36\% & 2.14\% & 338,182,510 & 135,694,739 \\
\hline I-70E Express Toll Lanes Scenario 1 & 934,026,910 & 394,877,422 & 20.00\% & 155,456,018 & 383,693,470 & 58.92\% & 10.00\% & 250,937,393 & 97,240,283 \\
\hline C-470 Express Toll Lanes Scenario 2 & 838,551,496 & 342,646,022 & 20.00\% & 139,815,126 & 356,090,347 & 57.54\% & 10.00\% & 208,463,594 & 85,359,182 \\
\hline Northwest Corridor Scenario 1 & 1,374,171,603 & 511,414,253 & 20.00\% & 230,175,248 & 632,582,101 & 53.97\% & 10.00\% & 286,314,081 & 120,009,645 \\
\hline Front Range Toll Road Express Toll Lanes Scenario 2 & 3,185,831,024 & 1,118,472,874 & 20.00\% & 535,043,465 & 1,532,314,685 & 51.90\% & 10.00\% & 645,201,105 & 271,532,386 \\
\hline 1-70 Mountain Corridor - Scenario 2 & 3,951,864,686 & 1,256,472,156 & 20.00\% & 666,449,664 & 2,028,942,867 & 48.66\% & 10.00\% & 610,475,250 & 268,215,850 \\
\hline Front Range Toll Road Express Toll Lanes Scenario 1 & 3,747,385,656 & 1,143,192,365 & 20.00\% & 632,981,471 & 1,971,211,821 & 47.40\% & 10.00\% & 675,303,371 & 284,841,439 \\
\hline Banning-Lewis Parkway Toll Road - Scenario 2 & 914,705,360 & 183,432,589 & 20.00\% & 156,517,093 & 574,755,678 & 37.16\% & 10.00\% & 134,309,112 & 54,563,285 \\
\hline Banning-Lewis Parkway Tollroad - Scenario 1 & 914,157,162 & 178,864,270 & 20.00\% & 156,517,093 & 578,775,799 & 36.69\% & 10.00\% & 138,215,349 & 54,660,250 \\
\hline U.S. 36 Express Toll Lanes & 1,894,262,095 & 326,263,957 & 20.00\% & 325,258,901 & 1,242,739,237 & 34.39\% & 10.00\% & 223,153,273 & 85,521,879 \\
\hline I-70 Mountain Corridor - Scenario 1 & 4,019,367,034 & 236,508,212 & 20.00\% & 699,744,728 & 3,083,114,094 & 23.29\% & 10.00\% & 225,471,439 & 76,317,237 \\
\hline U.S. 287 Corridor Express Toll Lanes & 222,093,398 & 3,542,420 & 20.00\% & 38,865,390 & 179,685,588 & 19.09\% & 10.00\% & 15,587,417 & 6,651,248 \\
\hline
\end{tabular}
=Denver Regional Area Projects Selected for Cashflow
\({ }^{(1)}\) Upfront transfers include federal moneys available in the form of a one-time, upfront payment
\({ }^{(2)}\) Gross transfers include the total annual state and local contributions over the life of the program
\({ }^{(3)}\) Net transfers are the present value at \(5.00 \%\) of the gross transfers to the year 2010

Table 6-6
Costs Associated with Entire Construction - Selected Alternatives, Market +100 bps
\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline \multirow[b]{2}{*}{Project} & Total 2010 & Senior Lien & \multicolumn{2}{|c|}{Federal \({ }^{(1)}\)} & \multirow[b]{2}{*}{Shortfall/ (Excess)} & \multirow[b]{2}{*}{\% of Project
Cost} & \multicolumn{3}{|c|}{State \& Local} \\
\hline & Project Cost with COI & Proceeds Par Amount & \[
\begin{gathered}
\text { Upfront } \\
\text { Transfers \% }
\end{gathered}
\] & Transfers \$ & & & \[
\begin{gathered}
\text { Annual } \\
\text { Transfers \% }
\end{gathered}
\] & \[
\begin{gathered}
\text { Gross } \\
\text { Transfers \$ }{ }^{(2)}
\end{gathered}
\] & \[
\begin{gathered}
\text { Net } \\
\text { Transfers \$ }{ }^{(3)}
\end{gathered}
\] \\
\hline 1-70E Express Toll Lanes Scenario 3 & 293,798,477 & 293,798,481 & 0.00\% & & (4) & 100.00\% & 0.00\% & & \\
\hline 1-70 Mountain Corridor - Scenario 3 & 1,097,605,975 & 1,097,602,625 & 0.00\% & & 3,350 & 100.00\% & 0.00\% & & - \\
\hline 1-25 Express Toll Lanes Scenario 1 & 512,469,481 & 441,904,339 & 0.00\% & - & 70,565,141 & 86.23\% & 0.00\% & & \\
\hline 1-225 Express Toll Lanes & 285,947,794 & 202,586,754 & 0.00\% & - & 83,361,040 & 70.85\% & 0.00\% & - & - \\
\hline 1-270 Express Toll Lanes & 341,301,223 & 208,666,022 & 10.80\% & 30,235,903 & 102,399,298 & 70.00\% & 0.00\% & & - \\
\hline C-470 Express Toll Lanes Scenario 2A & 852,245,201 & 490,245,955 & 15.21\% & 106,329,404 & 255,669,843 & 70.00\% & 0.00\% & - & - \\
\hline Powers Toll Road Scenario 4 & 394,171,598 & 211,258,891 & 20.00\% & 64,665,779 & 118,246,928 & 70.00\% & 0.88\% & 13,020,086 & 5,363,641 \\
\hline Northwest Corridor Scenario 2 & 526,511,457 & 282,154,424 & 20.00\% & 86,377,473 & 157,979,560 & 70.00\% & 7.00\% & 115,029,237 & 50,172,859 \\
\hline Denver Area Projects Scenario 2 & 4,760,118,716 & 2,457,452,967 & 20.00\% & 782,893,543 & 1,519,772,207 & 68.07\% & 10.00\% & 1,583,518,922 & 612,348,278 \\
\hline Denver Area Projects Scenario 1 & 4,706,535,729 & 2,245,619,933 & 14.41\% & 548,201,680 & 1,912,714,116 & 59.36\% & 2.14\% & 338,182,510 & 135,694,739 \\
\hline Front Range Toll Road Express Toll Lanes Scenario 1 & 3,747,385,656 & 1,143,192,365 & 20.00\% & 632,981,471 & 1,971,211,821 & 47.40\% & 10.00\% & 675,303,371 & 284,841,439 \\
\hline Banning-Lewis Parkway Toll Road - Scenario 2 & 914,705,360 & 183,432,589 & 20.00\% & 156,517,093 & 574,755,678 & 37.16\% & 10.00\% & 134,309,112 & 54,563,285 \\
\hline U.S. 36 Express Toll Lanes & 1,894,262,095 & 326,263,957 & 20.00\% & 325,258,901 & 1,242,739,237 & 34.39\% & 10.00\% & 223,153,273 & 85,521,879 \\
\hline U.S. 287 Corridor Express Toll Lanes & 222,093,398 & 3,542,420 & 20.00\% & 38,865,390 & 179,685,588 & 19.09\% & 10.00\% & 15,587,417 & 6,651,248 \\
\hline
\end{tabular}
= Denver Regional Area Projects Selected for Cashflow
\({ }^{(1)}\) Upfront transfers include federal moneys available in the form of a one-time, upfront payment
\({ }^{(2)}\) Gross transfers include the total annual state and local contributions over the life of the program
\({ }^{(3)}\) Net transfers are the present value at \(5.00 \%\) of the gross transfers to the year 2010


Figures 6-1 through 6-3 present a summary of the financial feasibility analyses previously discussed. Figure 6-1 presents project feasibility based on current market rates. Under this scenario, five projects are considered financially feasible in that 70 percent or more of project costs can be covered solely with toll revenue. Another four projects could be feasible with some federal funding support; the percent of federal funds ranging from 10.0 to 20.0 percent of the 70 percent feasibility threshold.

Figure 6-2 presents a similar summary of project feasibility but assumes an increase of 100 basis points over current market interest rates. This assumption produces similar results, with the exception of the I-270 Express Toll Lane project. With the increase in market interest rates, this project could be feasible if supported with 10.8 percent federal funds. The C-470, Scenario 2A project could also be feasible if supported with 15.2 percent federal funds. The other projects in this category could be feasible with the maximum 20.0 percent federal fund support.

Figure 6-3 provides a side-by-side comparison of both scenarios described above.
\begin{tabular}{|l|ccccl|}
\hline \multicolumn{1}{|c|}{ Project } & Type & Scenario & \begin{tabular}{c} 
Feasible \\
With Toll \\
Revenue Only
\end{tabular} & \begin{tabular}{c} 
Feasible \\
With Some \\
Federal Funds
\end{tabular} & \begin{tabular}{c} 
Not \\
Feasible
\end{tabular} \\
\hline I-25 North & ETL & 1 & & & \\
\hline I-70 East & ETL & 3 & & & \\
\hline U.S. Route 36 & ETL & 1 & & & \\
\hline I-225 & ETL & 1 & & & \\
\hline I-270 & ETL & 1 & & & \\
\hline C-470 & ETL & \(2 A\) & & & \\
\hline Northwest Corridor & Toll Road & 2 & & & \\
\hline Denver Area "System" & System & 1 & & & \\
\hline Denver Area "System" & System & 2 & & & \\
\hline I-70 Mountain Corridor & Tunnels & \(3 A\) & & & \\
\hline U.S. 287 Bypass & Toll Road & 1 & & & \\
\hline Powers Blvd. & Toll Road & 4 & & & \\
\hline Banning Lewis Pkwy & Toll Road & 2 & & & \\
\hline Front Range T.R. & Toll Road & 1 & & & \\
\hline
\end{tabular}
* Percent federal funds needed.

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\begin{tabular}{|l|ccccl|}
\hline \multicolumn{1}{|c|}{ Project } & Type & Scenario & \begin{tabular}{c} 
Feasible \\
With Toll \\
Revenue Only
\end{tabular} & \begin{tabular}{c} 
Feasible \\
With Some \\
Federal Funds
\end{tabular} & \begin{tabular}{c} 
Not \\
Feasible
\end{tabular} \\
\hline I-25 North & ETL & 1 & & & \\
\hline I-70 East & ETL & 3 & & & \\
\hline U.S. Route 36 & ETL & 1 & & & \\
\hline I-225 & ETL & 1 & & & \\
\hline I-270 & ETL & 1 & & & \\
\hline C-470 & ETL & \(2 A\) & & & \\
\hline Northwest Corridor & Toll Road & 2 & & & \\
\hline Denver Area "System" & System & 1 & & & \\
\hline Denver Area "System" & System & 2 & & & \\
\hline I-70 Mountain Corridor & Tunnels & \(3 A\) & & & \\
\hline U.S. 287 Bypass & Toll Road & 1 & & & \\
\hline Powers Blvd. & Toll Road & 4 & & & \\
\hline Banning Lewis Pkwy & Toll Road & 2 & & & \\
\hline Front Range T.R. & Toll Road & 1 & & & \\
\hline
\end{tabular}
* Percent federal funds needed.
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FINANCIAL FEASIBILITY SUMMARY (With Full Project Cost + 100bps)
\begin{tabular}{|c|c|c|c|c|}
\hline Project & Type & Scenario & Full Project Cost & \[
\begin{gathered}
\text { Full } \\
\text { Project } \\
\text { Cost }+100 \mathrm{bps}
\end{gathered}
\] \\
\hline I-25 North & ETL & 1 & - & - \\
\hline I-70 East & ETL & 3 & - & - \\
\hline U.S. Route 36 & ETL & 1 & \(\bigcirc\) & \(\bigcirc\) \\
\hline I-225 & ETL & 1 & - & - \\
\hline I-270 & ETL & 1 & - & (10.8\%) \\
\hline C-470 & ETL & 2A & (2.6\%) & (15.2\%) \\
\hline Northwest Corridor & Toll Road & 2 & (16.5\%) & (20.0\%) \\
\hline Denver Area "System" & System & 1 & \(\bigcirc\) & - \\
\hline Denver Area "System" & System & 2 & (19.4\%) & - \\
\hline I-70 Mountain Corridor & Tunnels & 3A & - & - \\
\hline U.S. 287 Bypass & Toll Road & 1 & \(\bigcirc\) & - \\
\hline Powers Blvd. & Toll Road & 4 & (10.0\%) & (20.0\%) \\
\hline Banning Lewis Pkwy & Toll Road & 2 & \(\bigcirc\) & - \\
\hline Front Range T.R. & Toll Road & 1 & \(\bigcirc\) & \(\bigcirc\) \\
\hline \multicolumn{2}{|r|}{- Feasible with Toll Revenue Only} & \multicolumn{3}{|l|}{Feasible with Tolls and Some Federal Funds Not Feasible
(Percent Federal Funds Needed)} \\
\hline
\end{tabular}

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\section*{Next Steps Toward Implementation}

This preliminary traffic and revenue study has tested the basic financial feasibility of utilizing tolls to finance the construction of transportation improvements in a wide range of corridors throughout Colorado. Although the results have indicated that a number of the corridors are potentially viable candidates for tolling, there is much work yet to be done before tolling could be implemented in any corridor. This chapter outlines those future tasks.

As illustrated by Figure 7-1, the next steps fall into two categories:
- Project Development; and
- Institutional Arrangements.


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In many cases, work on any number of these tasks could be on-going simultaneously; in some cases, certain tasks need to be completed before another task can even be initiated. These inter-relationships are also briefly described herein.

\section*{PROJECT DEVELOPMENT}

This category of future efforts involves elements that lead to the definition, the approval, and the design and construction of a toll facility in any of the corridors. The following are brief synopsis of the key tasks in this category.
- This study has suggested that defining a "system" approach to implementing toll facilities would likely be the most viable approach. System continuity of toll corridors is important, but even more critical is the approach of a financing system. The proper balance of the physical and the finance systems must be carefully considered.

It would likely be most strategic to first construct those projects or portions of projects that were found to have the highest financial feasibility or the ability to be self-supporting. Then, excess revenues generated by these early projects could help to fund those projects that are less viable, but that are still important components of the overall toll system from a system continuity and access standpoint.

If the CTE Board determines that this system approach is appropriate, a strategic definition of the system should be developed before any individual corridor proceeds into implementation.
- Prior to implementation of tolls in any corridor, those improvements will need to obtain environmental clearances through the National Environmental Policy Act process. Most of the corridors either have, or are currently being studied under the NEPA process, and coordination with these efforts should be on-going. These studies are in various stages of completion; some of the on-going studies are programmed to be completed within the next year, while others are at least several years from completion.

If this study indicates that a corridor is expected to be financially feasible for tolling, it is important that the NEPA study include a toll alternative and that it be carried forward as a reasonable alternative unless it has an environmental fatal flaw.

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It should be noted, however, that financial feasibility in this study does not imply that tolling will be the chosen alternative through the NEPA process, merely that it is a viable alternative which should be considered. Within the NEPA process, the secondary and indirect impacts of tolling, such as the impacts to alternative routes due to some traffic diversion, should be considered. Environmental impacts should not be significantly different from non-tolled alternatives because open-road tolling with no manual toll collection is being proposed on all projects. Considering these impacts up front in the NEPA documents can help streamline the environmental process and help to minimize the need for an environmental re-evaluation. If a Record of Decision is reached on an alternative that does not include tolls, the environmental clearances would need to be re-evaluated before a toll facility could be pursued.
- Each of the second-tier projects determined to be warranted for further consideration of tolling will need to be studied in more detail within an Investment Grade Study. An Investment Grade Study would include further optimization of toll rates, traveler origin-destination surveys, more detailed economic development analysis and further refinement of inputs into traffic models. In addition, more detailed capital and operating and maintenance cost estimates would be developed. The final Investment Grade Study would then be conducted at a level of detail suitable for pursuing actual toll project financing. A study of this nature typically requires 6 to 9 months to complete; ideally it would be programmed to be completed at the same time or before the environmental clearances for a project are obtained.
- A detailed financial plan will need to be prepared for each project. A process for additional funding will need to be determined for those projects that cover less than 100 percent of their capital costs through tolling. The toll revenues could be combined with a number of other funding mechanisms (federal, state or local).

\section*{INSTITUTIONAL ARRANGEMENTS}

Numerous options are available for the governance of a statewide system of express toll lanes or other toll projects. This group of next steps deals with the need to establish roles and responsibilities for the various entities who would be involved in implementation, the need for any legislative changes, and the organizational or structural needs of CTE in the future.

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- Because implementation will likely involve a number of players, it will be important to clearly define the role and responsibilities of each early in the process. The key players will be CDOT, CTE and potentially a wide range of outside service providers (general engineering consultant, traffic and revenue consultant, legal, bond counsel, financial advisor, and bond underwriter). Primary areas in which roles and responsibilities should be defined include:
- Operations and Maintenance - CTE will need to coordinate the operation and maintenance process with other entities, such as CDOT, other toll providers or even other private contractors, if they decide to out-source the operation and maintenance of the facility. It is important to note that a high level of maintenance is critical from a sales standpoint. Therefore, the maintenance provider must be able to ensure that the toll facility will always be a "priority" in their business operation.
- Back Office Functions - These functions are administrative in nature, with probably the most significant function being that of billing and collections. This could be performed by CTE, could be conducted under an agreement with another toll provider such as the E-470 Authority, or could be out-sourced to a private contractor.
- Right-of-Way/Construction - It will be important for CTE and CDOT to come to an agreement on how right-of-way and construction efforts will be handled. It is currently anticipated that CTE would lease right-of-way from CDOT. It is expected that CTE would be responsible for the construction of all toll facilities, while all "free" or general purpose lanes would remain the responsibility of CDOT. These, or other arrangements, should be institutionalized.
- The current legislation which enables the Colorado Tolling Enterprise, mandates that the new toll systems be interoperable with those systems which currently exist - namely E-470 and the Northwest Parkway. Thus, from a user's standpoint, the system would be "seamless" - one transponder, one bill, etc. Methods to ensure this interoperability should be considered and detailed.
- The existing legislation should be reviewed to ensure that the proposed toll projects meet the guidelines of the legislation. If necessary, appropriate revisions to the legislation should be proposed. If the I-70 west project proceeds, in a scenario in which tolls are applied to all traffic, it may be necessary to obtain a change in legislative authority
for CTE. Current enabling legislation does not permit tolling of existing capacity.
- Finally, dependent on the outcome of the legislative review and the other institutional arrangements, the structure and organization of CTE, as currently constituted, should be examined. Changes should be considered if they are necessary to efficiently manage and operate the toll system.

\section*{Appendix A}

Final Unit Cost Assumptions

\section*{Colorado}

Tolling Enterarise Board
\begin{tabular}{|c|c|c|c|}
\hline  & Units & \$/Unit or \% Construction & Item Description \\
\hline \multicolumn{4}{|l|}{Right of Way Acquisition} \\
\hline - Land - Undeveloped & Acre & \$ 10,000 & \multirow[t]{4}{*}{Cost of various types of Land and other right of way related items} \\
\hline - Land - Rural Residential Acreage & Acre & \$ 125,000 & \\
\hline - Land - Urban/Sub Residential Acreage & Acre & \$ 250,000 & \\
\hline - Land - Urban/Suburban Industrial (vacant) & Acre & \$ 40,000 & \\
\hline - Improvements - Rural Residential & L.S. Each & \$ 100,000 & Displacements taken by alignment \\
\hline - Improvements - Urban/Sub Residential & L.S. Each & \$ 250,000 & Displacements taken by alignment \\
\hline - Improvements - Comm./Indust. Urban/Sub & L.S. Each & \$ 800,000 & Displacements taken by alignment \\
\hline - Acquisition, Condemnation, and Admin. & \% & 40\% & Contingency shown as a \% of the Total R/W costs - Land + Improvements. \\
\hline - Relocation Contingency & \% & 25\% & Contingency applied to the costs for Improvements only. \\
\hline
\end{tabular}
\begin{tabular}{|l|l|l|l|}
\hline Utility Relocations & & & \\
\hline - Along Existing Alignment & Mile & \(\$ \$ 200,000\) & \begin{tabular}{l} 
Cost of relocating utilities for strategies that \\
widen or reconstruct a corridor, but not limited \\
to electric, gas, water, telephone, fiber optic, \\
pipelines, and sanitary sewers.
\end{tabular} \\
\hline & & & \begin{tabular}{l} 
Cost of relocating utilities for strategies that \\
build on new alignment including, but not \\
limited to electric, gas, water, telephone, fiber \\
optic, pipelines, and sanitary sewers.
\end{tabular} \\
- Along New Alignment & Mile & \(\$\) & 50,000
\end{tabular}
\begin{tabular}{|l|l|r|l|}
\hline Grading \& Drainage (Mainline) & & & \\
\hline \begin{tabular}{l} 
- Major Grading and Drainage: \\
Urban Highway Reconstruction 4 to 6 Lanes
\end{tabular} & CL Mile & \(\$ 1,300,000\) & includes grading and drainage \\
\hline \begin{tabular}{l} 
- Major Grading and Drainage: \\
Level, 4-Lane Highway on New Location
\end{tabular} & CL Mile & \(\$ 1,000,000\) & includes grading and drainage \\
\hline \begin{tabular}{l} 
- Major Grading and Drainage: \\
Rolling, 4-Lane Highway on New Location
\end{tabular} & CL Mile & \(\$ 3,000,000\) & includes grading and drainage \\
\hline - Erosion Control & CL Mile & \(\$ 200,000\) & Sodding, seeding, ditch paving, ditch checks, etc. \\
\hline - Fence & Lin. Ft. & \(\$ 7.50\) & \begin{tabular}{l} 
Used for new right-of-way only, both sides of \\
roadway
\end{tabular} \\
\hline
\end{tabular}
\begin{tabular}{|l|c|r|l|}
\hline Pavement, Shoulders \& Base & & & \\
\hline \begin{tabular}{l}
-Light Duty - 2-Lane Outer Roadway \\
Pavement and base
\end{tabular} & Sq. Yd. & \(\$ 40.00\) & \begin{tabular}{l} 
Cost for pavement, shoulders and base. (for use \\
on Outer Roadways) \\
Cost for pavement, shoulders and base.
\end{tabular} \\
\hline \begin{tabular}{l} 
- Heavy Duty -4 or 6-Lane Divided \\
Pavement and Base
\end{tabular} & Sq. Yd. & \(\$ 50.00\) & Lin. Ft. \\
\hline - Median Barrier & Sq. Yd. & \(\$ 50.00\) & \begin{tabular}{l} 
Cost for concrete median barrier and express toll \\
lane barriers, where applicable
\end{tabular} \\
\hline - Existing Pavement Removal & \(\$ 3.00\) & \begin{tabular}{l} 
Cost for removal and disposal of existing asphalt \\
and concrete pavement if required
\end{tabular} \\
\hline
\end{tabular}

Tolling Enterprise Board

\begin{tabular}{|l|c|r|l|l|}
\hline - 1-Lane Ramp (on structure) & Units & \begin{tabular}{c} 
Per Lin. \\
Foot of \\
Ramp
\end{tabular} & \(\$ 3,000\) & Cost for one-lane ramp on elevated structure. \\
\hline - 1-Lane Ramp (on earth) & \begin{tabular}{c} 
Per Lin. \\
Foot of \\
Ramp
\end{tabular} & \(\$ 270\) & Cost for one-lane ramp at grade. \\
\hline - Cross-road Reconstruction & \begin{tabular}{c} 
Per Lin. \\
Foot
\end{tabular} & \(\$ 600\) & \begin{tabular}{l} 
Cost to reconstruct cross-roads impacted by \\
roadway widening or reconstruction.
\end{tabular} \\
\hline - Typical Diamond Interchange (new) & Lump Sum & \(\$ 4,000,000\) & Cost for new interchange, excluding bridges \\
\hline - Typical Diamond Interchange (reconstruction) & Lump Sum & \(\$ 6,500,000\) & \begin{tabular}{l} 
Cost for reconstructing an existing interchange, \\
excluding bridges.
\end{tabular} \\
\hline - Lighting & Per Intch. & \(\$ 550,000\) & \begin{tabular}{l} 
Cost for full interchange lighting (based on \\
diamond configuration)
\end{tabular} \\
\hline
\end{tabular}
\begin{tabular}{|c|c|c|c|}
\hline Bridges/Structures & & & \\
\hline - Roadway over/under & Sq. Ft. & \$100 & Costs for construction of a roadway over or under an existing roadway. \\
\hline - Railroad over & Lin. Ft. & \$10,000 & Costs for construction of a single track railroad bridge over roadway corridor \\
\hline - Elevated Roadway Structure & Sq. Ft. & \$100 & Costs for construction of an elevated roadway over or parallel to an existing roadway corridor. \\
\hline - Bridge Removal & EA & \$20,000 & Costs for removal of existing bridges \\
\hline - Retaining Wall & \[
\begin{gathered}
\hline \text { Per Sq. Ft. } \\
\text { of Face }
\end{gathered}
\] & \$50 & Cost of miscellaneous wall usage based on front face \\
\hline
\end{tabular}
\begin{tabular}{|c|c|c|c|}
\hline Miscellaneous & & & \\
\hline - Maintenance of traffic during construction & \begin{tabular}{l}
\% of Total \\
Roadway \\
Costs \\
Excluding \\
Bridges
\end{tabular} & 4\% & Costs for all traffic items to construct project including, temporary median barrier, impact attenuators, pavement marking, signs, lighting, traffic signals, message boards, flashing arrow boards, barricades, reflectorized drums, and other channelizing devices, applied to construction costs, excluding bridge costs. \\
\hline - Mobilization & \% of Construct. Costs & 5\% & Contractor Mobilization Costs - \% of total construction costs only \\
\hline - Traffic Control & \begin{tabular}{l}
\% of Total \\
Roadway \\
Costs \\
Excluding \\
Bridges
\end{tabular} & 4\% & Costs for signage, pavement marking, special toll lane markings, buffers, way-finding, etc., applied to construction costs, excluding bridge costs. \\
\hline - Construction Inspection & \% of Construct. Costs & 17\% & Costs for inspection of construction projects, to include cost for construction staking. Construction costs only. \\
\hline
\end{tabular}

Tolling Enterprise Board
\begin{tabular}{|c|c|c|c|}
\hline & Units & \$/Unit or \% Construction & Item Description \\
\hline - Engineering - Design & \% of Construct. Costs & 8\% & Design of all items, including detailed design and plans, right of way strip maps and descriptions, geotechnical investigations, surveying, aerial mapping, and utility relocation coordination, construction costs and other misc items \\
\hline - Program Management and Administration & \% of Construct. Costs & 4\% & cost of administration of project, applied to construction costs and other misc. items \\
\hline - Contingency & \% of Construct. Costs & 20\% & Contingency funds applied to construction costs \\
\hline
\end{tabular}```


[^0]:    ${ }^{(1)}$ Not adjusted for ramp-up.
    ${ }^{(2)}$ Uninflated, in constant 2004 dollars.

[^1]:    ${ }^{(1)}$ Not adjusted for ramp-up.
    ${ }^{(2)}$ Uninflated, in constant 2004 dollars.

[^2]:    ${ }^{(1)}$ Not adjusted for ramp-up.
    ${ }^{(2)}$ Uninflated, in constant 2004 dollars.

[^3]:    ${ }^{(1)}$ Not adjusted for ramp-up.
    ${ }^{(2)}$ Uninflated, in constant 2004 dollars.

[^4]:    ${ }^{(1)}$ Not adjusted for ramp-up.
    ${ }^{(2)}$ Uninflated, in constant 2004 dollars.

[^5]:    ${ }^{(1)}$ Not adjusted for ramp-up.
    ${ }^{(2)}$ Uninflated, in constant 2004 dollars.

[^6]:    ${ }^{(1)}$ Not adjusted for ramp-up.
    ${ }^{(2)}$ Uninflated, in constant 2004 dollars.

[^7]:    ${ }^{(1)}$ Not adjusted for ramp-up.
    ${ }^{(2)}$ Uninflated, in constant 2004 dollars.

[^8]:    ${ }^{(1)}$ Not adjusted for ramp-up.
    ${ }^{(2)}$ Uninflated, in constant 2004 dollars.

[^9]:    ${ }^{(1)}$ Not adjusted for ramp-up.

[^10]:    ${ }^{(1)}$ Not adjusted for ramp-up.

[^11]:    ${ }^{(1)}$ Not adjusted for ramp-up.

[^12]:    ${ }^{(1)}$ Not adjusted for ramp-up.

[^13]:    ${ }^{(1)}$ Not adjusted for ramp-up.

[^14]:    ${ }^{(1)}$ Not adjusted for ramp-up.

[^15]:    ${ }^{(1)}$ Not adjusted for ramp-up.

[^16]:    ${ }^{(1)}$ Not adjusted for ramp-up.

