

COLORADO DEPARTMENT OF TRANSPORTATION



No Parking Sign Guide

2024 Edition

ACKNOWLEDGEMENTS

AASHTO A Policy on Geometric Design of Highways and Streets

FHWA Manual on Uniform Traffic Control Devices (2023).

Esayas Butta, CDOT HQ Traffic Safety & Engineering Services

Anthony Vu, CDOT HQ Traffic Safety & Engineering Services

Nathan Rivera, CDOT HQ Traffic Safety & Engineering Services

Yesenya Saucedo Paez, CDOT HQ Traffic Safety & Engineering Services

Alvin Stamp, CDOT Region 1

Azeb Seifu, CDOT Region 1

Christiana Lacombe, CDOT Region 1

Jocelyn Higashide, CDOT Region 1

Matt Jagow, CDOT Region 2

Pepper Whittlef, CDOT Region 2

James Biren, CDOT Region 2

Andi Staley, CDOT Region 3

Mark Bunnell, CDOT Region 3

Bryce Reeves, CDOT Region 4

Jonathan Woodworth, CDOT Region 4

James Lambrecht, CDOT Region 4

David Peyton, CDOT Region 5



Published on March 1, 2024

HQ Traffic Safety and Engineering Branch
2829 West Howard Place, 4th Floor

Denver, CO 80204

Telephone: (303) 757-9654 Fax: (303) 757-9219

INTRODUCTION

The NO PARKING ANY TIME (R7-1) and no parking symbol (R8-3) signs are an optional addition to the Colorado State Highway to reinforce sections where parking and/or stopping is prohibited. The Manual on Uniform Traffic Control Devices (MUTCD) does not have specific guidance on regulations surrounding signs that govern parking and stopping. The Model Traffic Code for Colorado prohibits stopping, standing, or parking along every highway, street, or roadway in respect to which owners or occupants of abutting lands and other persons have no legal right of access, and at other locations as found in C.R.S. 42-4-1204. The Colorado Department of Transportation (CDOT) has chosen to work with local agencies to allow a limited use of the R7-1 and R8-3 signs on Colorado State Highways. This document is intended to serve as a guide for the use and placement of the R7-1 and R8-3 signs. The local agency shall submit the request form in Appendix C to the corresponding CDOT Region Traffic Engineer (RTE) for review. The CDOT RTE will review the information provided from the local agency and determine whether signage is warranted.

SIGN GUIDANCE

Use of the R7-1 on State Highways are allowable only when a written agreement from local law enforcement that signs will be enforced and at least one (1) of the following criteria are met:

- The location is near a hazardous area such as an avalanche zone or rockslide area.
- The sight distance provided is less than the calculated sight distance' from the latest edition of the American Association of State Highway and Transportation Officials (AASHTO) A Policy Geometric Design of Highways and Streets. 1
- A nearby attraction that would prompt travelers to park at the location.
- A pattern of parking related crashes the past 3 years in urban areas and the past 5 years in rural areas.
- Insufficient width of usable and paved shoulder at the location.
- The calculated intersection sight distance is impacted.²
- or at CDOT's RTE discretion for circumstances not covered above

When criteria is met, sign R7-1(L) may be installed at the beginning and R7-1(R) may be installed at the end of the no parking zone. For locations that are longer than 200' it is recommended that additional R8-3 signs may be installed at 200' intervals.

R7-2a may be used at the discretion of the RTE when the criteria is met for specific times of the day.

The final determination of whether or not to install no parking signs lies with the CDOT RTE.











R8-3

¹ See Appendix A for tables on stopping sight distance on level roads and grades from A Policy Geometric Design of Highways and Streets.

² See Appendix A for tables on intersection sight distance on level roads and grades from A Policy Geometric Design of Highways and Streets.

APPENDIX A: STOPPING SIGHT DISTANCE TABLES

The source of all the tables in Appendix A is AASHTO's *A Policy on Geometric Design of Highways and Streets*.

Table 3-1. Stopping Sight Distance on Level Roadways

	U.S. Customary					Metric			
Design Speed	Brake Reaction	Braking Distance	Stopp Sight Dis	_	Design Speed	Brake Reaction	Braking Distance	Stopp Sight Dis	_
(mph)	Distance (ft)	on Level (ft)	Calculated (ft)	Design (ft)	(km/h)	Distance (m)	on Level (m)	Calculated (m)	Design (m)
15	55.1	21.6	76.7	80	20	13.9	4.6	18.5	20
20	73.5	38.4	111.9	115	30	20.9	10.3	31.2	35
25	91.9	60.0	151.9	155	40	27.8	18.4	46.2	50
30	110.3	86.4	196.7	200	50	34.8	28.7	63.5	65
35	128.6	117.6	246.2	250	60	41.7	41.3	83.0	85
40	147.0	153.6	300.6	305	70	48.7	56.2	104.9	105
45	165.4	194.4	359.8	360	80	55.6	73.4	129.0	130
50	183.8	240.0	423.8	425	90	62.6	92.9	155.5	160
55	202.1	290.3	492.4	495	100	69.5	114.7	184.2	185
60	220.5	345.5	566.0	570	110	76.5	138.8	215.3	220
65	238.9	405.5	644.4	645	120	83.4	165.2	248.6	250
70	257.3	470.3	727.6	730	130	90.4	193.8	284.2	285
75	275.6	539.9	815.5	820	140	97.3	224.8	322.1	325
80	294.0	614.3	908.3	910					
85	313.5	693.5	1007.0	1010					

Note: Brake reaction distance predicated on a time of 2.5 s; deceleration rate of 11.2 ft/s^2 [3.4 m/s^2] used to determine calculated sight distance.

Table 3-2. Stopping Sight Distance on Grades

		0.000											
	U.S. Customary							N	letric				
Design		Stoppi	ng Sigh	t Dista	nce (ft)		Design	gn Stopping Sight Distance (m)					
Speed	Do	wngrad	des	U	pgrade	es	Speed	Do	wngra	des	U	pgrade	es
(mph)	3%	6%	9%	3%	6%	9%	(km/h)	3%	6%	9%	3%	6%	9%
15	80	82	85	75	74	73	20	20	20	20	19	18	18
20	116	120	126	109	107	104	30	32	35	35	31	30	29
25	158	165	173	147	143	140	40	50	50	53	45	44	43
30	205	215	227	200	184	179	50	66	70	74	61	59	58
35	257	271	287	237	229	222	60	87	92	97	80	77	75
40	315	333	354	289	278	269	70	110	116	124	100	97	93
45	378	400	427	344	331	320	80	136	144	154	123	118	114
50	446	474	507	405	388	375	90	164	174	187	148	141	136
55	520	553	593	469	450	433	100	194	207	223	174	167	160
60	598	638	686	538	515	495	110	227	243	262	203	194	186
65	682	728	785	612	584	561	120	263	281	304	234	223	214
70	771	825	891	690	658	631	130	302	323	350	267	254	243
75	866	927	1003	772	736	704	140	341	367	398	302	287	274
80	965	1035	1121	859	817	782							
85	1070	1149	1246	949	902	862							

APPENDIX B: INTERSECTION SIGHT DISTANCE TABLES

The source of all the tables in Appendix A is AASHTO's *A Policy on Geometric Design of Highways and Streets*.

Table 9-4. Length of Sight Triangle Leg—Case A, No Traffic Control

U.S. Cu	U.S. Customary				
Design Speed (mph)	Length of Leg (ft)				
15	70				
20	90				
25	115				
30	140				
35	165				
40	195				
45	220				
50	245				
55	285				
60	325				
65	365				
70	405				
75	445				
80	485				

Metric				
Design Speed (km/h)	Length of Leg (m)			
20	20			
30	25			
40	35			
50	45			
60	55			
70	65			
80	75			
90	90			
100	105			
110	120			
120	135			
130	150			

Note: For approach grades greater than 3 percent, multiply the sight distance values in this table by the appropriate adjustment factor from Table 9-5.

Table 9-7. Design Intersection Sight Distance—Case B1, Left Turn from Stop

U.S. Customary					
Design Speed	Stopping Sight	Intersection Sight Distance for Passenger Cars			
(mph)	Distance (ft)	Calculated (ft)	Design (ft)		
15	80	165.4	170		
20	115	220.5	225		
25	155	275.6	280		
30	200	330.8	335		
35	250	385.9	390		
40	305	441.0	445		
45	360	496.1	500		
50	425	551.3	555		
55	495	606.4	610		
60	570	661.5	665		
65	645	716.6	720		
70	730	771.8	775		
75	820	826.9	830		
80	910	882.0	885		

	Metric					
Design Speed	Stopping Sight Distance	Intersection Sight Distance for Passenger Cars				
(km/h)	(m)	Calculated (m)	Design (m)			
20	20	41.7	45			
30	35	62.6	65			
40	50	83.4	85			
50	65	104.3	105			
60	85	125.1	130			
70	105	146.0	150			
80	130	166.8	170			
90	160	187.7	190			
100	185	208.5	210			
110	220	229.4	230			
120	250	250.2	255			
130	285	271.1	275			

Note: Intersection sight distance shown is for a stopped passenger car to turn left onto a two-lane highway with no median and grades 3 percent or less. For other conditions, the time gap should be adjusted and the sight distance recalculated.

Table 9-9. Design Intersection Sight Distance—Case B2, Right Turn from Stop

U.S. Customary					
Design Speed (mph)	Stopping Sight Distance	Intersection Sight Distance for Passenger Cars			
	(ft)	Calculated	Design		
		(ft)	(ft)		
15	80	143.3	145		
20	115	191.1	195		
25	155	238.9	240		
30	200	286.7	290		
35	250	334.4	335		
40	305	382.2	385		
45	360	430.0	430		
50	425	477.8	480		
55	495	525.5	530		
60	570	573.3	575		
65	645	621.1	625		
70	730	668.9	670		
75	820	716.6	720		
80	910	764.4	765		

Metric					
Design Speed (km/h)	Stopping Sight Distance	Intersecti Distand Passeng	ce for		
	(m)	Calculated (m)	Design (m)		
20	20	36.1	40		
30	35	54.2	55		
40	50	72.3	75		
50	65	90.4	95		
60	85	108.4	110		
70	105	126.5	130		
80	130	144.6	145		
90	160	162.6	165		
100	185	180.7	185		
110	220	198.8	200		
120	250	216.8	220		
130	285	234.9	235		

Note: Intersection sight distance shown is for a stopped passenger car to turn right onto or to cross a two-lane roadway with no median and with grades of 3 percent or less. For other conditions, the time gap should be adjusted and the sight distance recalculated.

Table 9-11. Design Intersection Sight Distance—Case B3, Crossing Maneuver

U.S. Customary						
Design Speed (mph)	Stopping Sight Distance	Intersection Sight Distance for Passenger Cars				
	(ft)	Calculated	Design			
		(ft)	(ft)			
15	80	143.3	145			
20	115	191.1	195			
25	155	238.9	240			
30	200	286.7	290			
35	250	334.4	335			
40	305	382.2	385			
45	360	430.0	430			
50	425	477.8	480			
55	495	525.5	530			
60	570	573.3	575			
65	645	621.1	625			
70	730	668.9	670			
75	820	716.6	720			
80	910	764.4	765			

Metric					
Stopping Sight Distance	Distan	ce for			
(m)	Calculated (m)	Design (m)			
20	36.1	40			
35	54.2	55			
50	72.3	75			
65	90.4	95			
85	108.4	110			
105	126.5	130			
130	144.6	145			
160	162.6	165			
185	180.7	185			
220	198.8	200			
250	216.8	220			
285	234.9	235			
	Stopping Sight Distance (m) 20 35 50 65 85 105 130 160 185 220 250	Stopping Sight Intersecti Distance Passeng (m) Calculated (m) 20 36.1 35 54.2 50 72.3 65 90.4 85 108.4 105 126.5 130 144.6 160 162.6 185 180.7 220 198.8 250 216.8			

Note: Intersection sight distance shown is for a stopped passenger car to turn right onto or to cross a two-lane roadway with no median and with grades of 3 percent or less. For other conditions, the time gap should be adjusted and the sight distance recalculated.

Table 9-15. Design Intersection Sight Distance—Case C2, Left or Right Turn at Yield-Controlled Intersections

U.S. Customary					
	Stopping	Length of Leg			
Design Speed	Sight	Passenger Cars			
(mph)	Distance (ft)	Calculated (ft)	Design (ft)		
15	80	176.4	180		
20	115	235.2	240		
25	155	294.0	295		
30	200	352.8	355		
35	250	411.6	415		
40	305	470.4	475		
45	360	529.2	530		
50	425	588.0	590		
55	495	646.8	650		
60	570	705.6	710		
65	645	764.4	765		
70	730	823.2	825		
75	820	882.0	885		
80	910	940.8	945		

Metric						
	Stopping	Length of Leg Passenger Cars				
Design Speed	Sight					
(km/h)	Distance (m)	Calculated (m)	Design (m)			
20	20	44.5	45			
30	35	66.7	70			
40	50	89.0	90			
50	65	111.2	115			
60	85	133.4	135			
70	105	155.7	160			
80	130	177.9	180			
90	160	200.2	205			
100	185	222.4	225			
110	220	244.6	245			
120	250	266.9	270			
130	285	289.1	290			

Note: Intersection sight distance shown is for a passenger car making a right or left turn without stopping onto a two-lane road.

Table 9-17. Intersection Sight Distance—Case F, Left Turn from the Major Road

U.S. Customary				
Design Speed (mph)	Stopping Sight Distance (ft)	Intersection Sight Distance		
		Passenger Cars		
		Calculated (ft)	Design (ft)	
15	80	121.3	125	
20	115	161.7	165	
25	155	202.1	205	
30	200	242.6	245	
35	250	283.0	285	
40	305	323.4	325	
45	360	363.8	365	
50	425	404.3	405	
55	495	444.7	445	
60	570	485.1	490	
65	645	525.5	530	
70	730	566.0	570	
75	820	606.4	610	
80	910	646.8	650	

Metric				
Design Speed (km/h)	Stopping Sight Distance (m)	Intersection Sight Distance		
		Passenger Cars		
		Calculated (m)	Design (m)	
20	20	30.6	35	
30	35	45.9	50	
40	50	61.2	65	
50	65	76.5	80	
60	85	91.7	95	
70	105	107.0	110	
80	130	122.3	125	
90	160	137.6	140	
100	185	152.9	155	
110	220	168.2	170	
120	250	183.5	185	
130	285	198.8	200	

Note: Intersection sight distance shown is for a passenger car making a left turn from an undivided roadway. For other conditions and design vehicles, the time gap should be adjusted and the sight distance recalculated.

APPENDIX C: REQUEST FORM

COLORADO DEPARTMENT OF TRANSPORTATION No Parking Sign Request Form

Local Agency Representative Contact Information					
Name:	Email:	Phone Number:			
Location Description					
County/City:	ounty/City: Speed Limit:				
State Highway:		End Milepost:			
	Site Description	on			
List all nearby attractions that may prompt someone to park:					
	area (e.g., avalanche, rockslide area, etc.)?	Yes No			
Available Sight Distance	t Distance: Calculated Sight Distance ¹ :				
Shoulder Width:					
	Additional Conside	rations			
	CDOT USE ON	LY			
Does maintenance see any concerns with parking at this location?					
Do the number of parking related crashes exceed what is expected for the past 3 years in urban areas or past 5 years in rural areas? Yes No					

¹ The method for calculated site distance shall be in accordance with AASHTO's *A Policy on Geometric Design of Highways and Streets*.